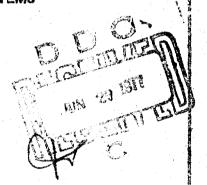
EAVID W. TAYLOR NAVAL SHIP -SEARCH AND DEVELOPMENT CENTER

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A FORTRAN IV COMPUTER PROGRAM FOR THE TIME DOMAIN ANALYSIS
OF THE TWO-DIMENSIONAL DYNAMIC MOTIONS
OF GENERAL BUOY-CABLE-BODY SYSTEMS

Henry T. Wang



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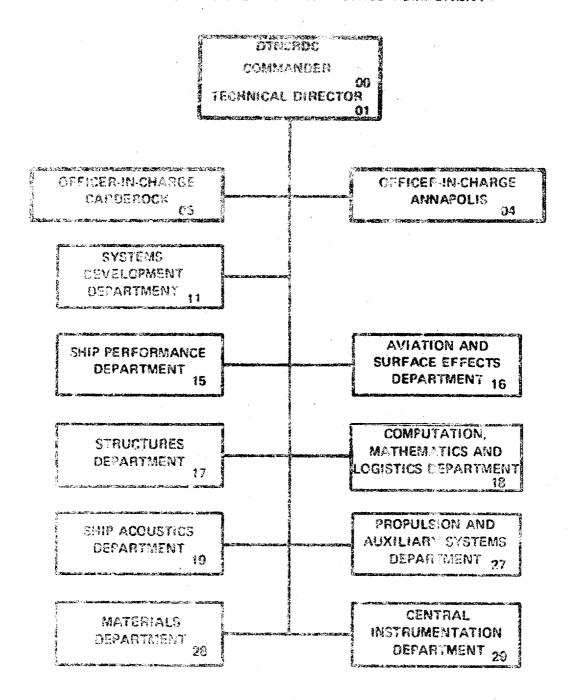
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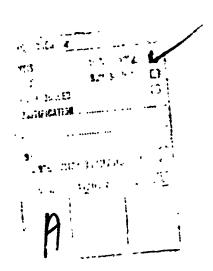
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ABSTRACT

The present report gives a detailed description of Program CABUOY, which analyzes in the time domain the two dimensional dynamic behavior of general ocean cable systems consisting of a surface buoy, connecting cable, and intermediate bodies. The equations which model the motions of the surface waves and the various components of the cable system are presented, and the subroutines of the program are briefly outlined. Instructions on use of the program include a listing of the input READ statements, definitions of the input variables, and a number of comments on the entering of input data. Several sample problems are given to illustrate use of the program, the output of the program, and computer costs for a range of cases. The listing of the program is given in the appendix.

ADMINISTRATIVE INFORMATION

The work described in this report was authorized by the Naval Air Development Center under Project Orders 4-0601 and 0-0611 respectively dated 5 March 1974 and 13 February 1976. The work was performed under internal Work Units 1-1552-130 and 1-1552-145.

INTRODUCTION

The dynamic motion characteristics of cable systems are currently of extreme interest, and several major surveys of cable dynamics studies have been made in recent years 1-3

An ocean cable system generally consists of the following three components:

- 1. A ship or surface float at the upper end.
- 2. A cable whose properties may vary along its length,
- 3. Intermediate bodies along the cable, including the possibility of a body at the lower end, Previous studies have usually focused on only one of the above components. For example, the principal emphasis in many studies is on the dynamic characteristics of the cable itself.

¹ Dillon, D.B., "An Inventory of Current Mathematical Models of Scientific Data - Gathering Moors," Hydrospace-Challenger, Inc. TR 4450 0001 (Feb 1973).

A complete list of references is given on pages 61-63.

²Choo, Y.I. and M.J. Casarella, "A Survey of Analytical Methods for Dynamic Simulation of Cable-Body Systems," Journal of Hydronautics, Vol. 7, No. 4, pp. 137-144 (Oct 1973).

³Albertsen, N.D., "A Survey of Techniques for the Analysis and Design of Submerged Mooring Systems," Civil Engineering Laboratory Technical Report R-815 (Aug 1974).

Conditions at the ends of the cable are then either those of prescribed motions or simple representations of the surface buoy or lower body. It appears that such studies were carried out mainly to demonstrate the feasibility of a particular method of solving for the dynamic characteristics of the cable. Choo and Casarella² discuss the merits and drawbacks of the three principal analytical methods: linearized frequency-domain method, method of characteristics, and finite element method. In other studies, the principal emphasis is on the dynamic characteristics of the surface buoy or the lower body and the effect of the cable is then approximated in various ways. It is clear that these studies are suitable only for analyzing particular types of cable systems, also, only the dynamic characteristics of certain components are accurately described.

The present report gives details on Program CABUOY, which analyzes in the time domain the two-dimensional dynamic behavior of all three components of a general cable system. This program has already been briefly described.⁴ Although it was developed principally to analyze the dynamic behavior of sonobuoy systems, for which it is of interest to know the dynamic behavior of the surface buoy, connecting cable, and lower acoustic detection units, the great generality and versatility of the program make it useful for a wide variety of other cable systems.

The report first presents in detail the equations which form the basis of the program. These include equations for the steady-state cable configuration and for the dynamic motions of the surface waves, surface buoy, cable, and intermediate bodies. Each of the program subroutines is briefly described. Detailed input instructions include a listing of the input FORTRAN READ statements, definition of the input variables contained in these READ statements, and comments on the entry of input data. Several sample problems serve to illustrate program use, output, and computer costs. The program is listed in the appendix.

Both the input instructions and the sample problems illustrate the wide applicability of the program. The sample problems range from a parametric study of the accuracy and computer cost of various finite element representations of the cable to the analysis of a complete buoy-cable-body system moored in the presence of typical ocean waves and current profiles. The characteristics of each component of the ocean surface waves may be specified by the user or may be internally generated by the program by means of the Picrson-Moskowitz spectrum. The surface buoy at the top of the cable may have a relatively wide range of sizes and shapes. It may be a prolate or oblate spheroid of any aspect ratio provided its horizontal

Wang, H.T., "Preliminary Report on a Fortran IV Computer Program for the Two-Dimensional Dynamic Behavior of General Ocean Cable Systems," DTNSRDC Departmental Report SPD-633-01 (Aug 1975).

length is small compared to the wavelengths of the significant ocean waves, or it may be a spar buoy of any size. The reasons for these particular choices of buoy shapes and sizes are given in the section on surface buoys. Alternatively, motions may be prescribed at the upper end. The user determines the accuracy and computer cost of the dynamic analysis of the cable by specifying the total number of cable segments as well as the length of each segment. Several different formulations are given for the added masses and drag coefficients of the intermediate bodies.

STEADY-STATE CALCULATIONS

CABLE EQUATIONS

The program first calculates the configuration of the cable system in the presence of a steady-state current* alone, in the absence of any time-dependent excitations. The differential equations for the steady-state configuration of the cable are well known and take the following form for the coordinate system shown in Figure 1; for example, see Springston.⁵

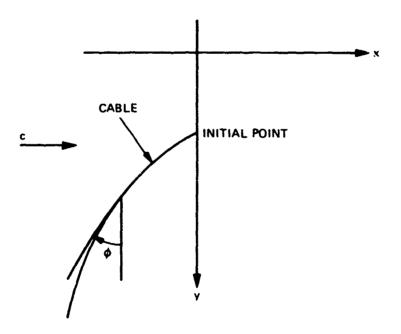


Figure 1 - Definition of Coordinate System

^{*}The term current is used to denote the steady-state fluid velocity relative to the cable. For the case of a cable towed in still water, the current has magnitude equal to and direction opposite to the towing velocity.

⁵Springston, G.B. Jr., "Generalized Hydrodynamic Loading Functions for Bare and Faired Cables in Two-Dimensional Steady-State Cable Configurations," NSRDC Report 2424 (Jun 1967).

$$-T \frac{d\phi}{ds_0} + I + \sin \phi W = 0$$
 (1)

$$\frac{dT}{ds_0} + G + \cos \phi W = 0 \tag{2}$$

$$\frac{ds}{ds_0} = 1 + \epsilon \tag{3}$$

$$\frac{\mathrm{dx}}{\mathrm{ds}_{\Omega}} - (1 + \epsilon) \sin \phi \tag{4}$$

$$\frac{\mathrm{d}y}{\mathrm{d}s_0} = (1 + \epsilon)\cos\phi \tag{5}$$

where T = cable tension

 ϕ = angle of the cable segment with the vertical (see Figure 1)

 s_0 = reference cable length (when T = T_0) measured from the initial point

 $T_0 = reference tension$

I.G = normal and tangential drag forces, respectively, per unit length acting on the caple

W = weight in fluid per unit length of the cable

s = stretched cable length measured from the initial point

 ϵ = cable strain; ϵ = 0 at T = T₀

x = horizontal displacement, positive to the right

y = vertical displacement, positive downward

For smooth, approximately round cables, the normal and tangential drags may be taken as respectively proportional to the squares of the velocities normal and tangential to the cable⁵

$$I = \frac{1}{2} \rho C_D d c_n |c_n|$$
 (6)

$$G = \frac{1}{2} \rho C_T d c_t |c_t|$$
 (7)

where ρ = fluid density

 $C_D, C_T = \text{normal}$ and tangential drag coefficients respectively

d = cable diameter

 $c_n = component of the current normal to the cable = c cos <math>\phi$

 c_t = component of the current tangential to the cable = $-c \sin \phi$

e = magnitude of the current, taken to act only in the x-direction

The tension-strain function is assumed to be of the form⁶

$$T - T_o = C_1 \epsilon^{C_2}$$
 (8)

where $C_1 = \text{constant of elasticity}$; $C_1 = AE$ for a linearly elastic cable

A = cross-sectional area = $\pi d^2/4$ for a round cable

E = modulus of elasticity

 C_2 = an exponent: C_2 = 1 for a linearly elastic cable

Equation (8) enables a nonlinear tension-strain relation to be modeled by only two input variables, C_1 and C_2 . It is more convenient to express ϵ as a function of $(T - T_{c_1})$ in order to eliminate it in Equations (3) through (5):

$$\epsilon = \left(\frac{T - T_o}{C_1}\right)^{1/C_2} \tag{9}$$

INTERMEDIATE BODIES

It is assumed that conditions at the top of the cable are known and the integration of Equations (1) through (5) proceeds down the cable. At an intermediate body, the integration must be interrupted and the unknown cable variables T_u and ϕ_u below the body must be related to the known variables T_k and ϕ_k above the body. Reducing the three-dimensional equations contained in References 7 and 8 to the present two-dimensional case results in the following two equations for T_u and ϕ_u :

$$T_{ii} = \sqrt{(\sin \phi_k T_k + D_x)^2 + (-\cos \phi_k T_k + W_B)^2}$$
 (10)

$$\phi_{u} = \tan^{-1} \frac{\sin \phi_{k} T_{k} + D_{x}}{\cos \phi_{k} T_{k} + W_{B}}$$
 (11)

where D_x is the drag on the body and

⁶Thresher, R.W. and J.H. Nath, "Anchor-Last Deployment Procedure for Mooring," Oregon State University Report 73-5 (Jun 1973).

⁷Wang, H.T., "Effect of Nonplanar Current Profiles on the Configuration of Moored Cable Systems," NSRDC Report 3692 (Oct 1971).

⁸Wang, H.T., "A FORTRAN IV Program for the Three-Dimensional Steady-State Configuration of Extensible Flexible Cable Systems," NSRDC Report 4384 (Sep 1974).

$$D_{\chi} = \frac{1}{2} \rho C_D A_{\chi} c(c)$$
 (12)

Here also, $C_D A_X$ is the drag area of the body for flow in the x-direction and W_B is the weight of the body in fluid.

BOUNDARY CONDITIONS

Initial Value Cases

The integration of the above differential equations is most convenient when the tension. I and the angle ϕ are known at one end of the cable. These are known for certain cases of single-point moored cables and towing cables. For the moored cases, the program starts with the known conditions at the top of the cable and integrates the five differential equations until the lower end of the cable is reached. This is the simplest case for the program since the numbering of the cable segments and intermediate bodies starts at the top of the cable. For towing cable cases, where the conditions are known at the lower towed body, the program integrates the differential equations twice. They are first integrated from the towed body to the upper point, thus fixing the conditions at this point. Then, in order to conform to the numbering system which is used for the dynamic calculations, the equations are integrated once again from the upper point down to the lower towed body.

Boundary Value Cases

In many applications, the values of I and ϕ are not known a priori at any point along the cable. It is necessary to treat these cases as boundary value problems and use iteration techniques to obtain the solution. The present program contains the iteration schemes for two cases of particular interest for onobuoy systems, a cable of given length moored in a given ocean depth⁹ and a tree-floating cable system. The iteration subroutine is written so that the user may conveniently implyment iteration schemes for other applications.

⁹Wang, H.T. and B.L. Webster, "Current Profiles Which Give Rise to Nonunique Solutions of Moored Cable Systems," Paper OTC 1538, Fourth Annual Offshore Technology Conference, Houston, Texas (May 1972).

¹⁰Wang, H.T. and T.L. Moran, "Analysis of the Two-Dimensional Steady-State Behavior of Extensible Free-Floating Cable Systems," NSRDC Report 3721 (Oct 1971)

OCEAN SURFACE WAVES

DESCRIPTION OF MOTION

For ocean depths greater than one-half the wavelength, ¹¹ the water particle trajectories due to a single progressive wave are, according to linearized first-order theory, given by ^{11,12}

$$x_{w} = a_{w} e^{-ky} \cos(kx - \sigma t + \theta_{w})$$
 (13a)

$$y_w = -a_w e^{-ky} \sin(kx - \sigma t + \theta_w)$$
 (13b)

where $x_w, y_w =$ water particle displacements in the (x, y) directions, respectively

a_w = wave amplitude

k = wave number = $2\pi/\lambda$

 λ = wavelength

 σ = circular frequency = $\sqrt{2\pi g/\lambda}$ = $2\pi f$

f = frequency

t = time

g = gravity constant = $32.2 \text{ ft/sec}^2 (9.81 \text{ m/sec}^2)$

 θ_{w} = phase angle

It is of interest to note that the trajectories describe circular orbits with a radius which decays exponentially with depth.

For an irregular sea consisting of N distinct components, the resultant water particle displacements are obtained by a summation of the above expressions, resulting in

$$x_{w} = \sum_{i=1}^{N} a_{wi} e^{-k_{i}y} \cos(k_{i}x - \sigma_{i}t + \theta_{wi})$$
 (14a)

$$y_{w} = -\sum_{i=1}^{N} a_{wi} e^{-k_{i}y} \sin(k_{i}x - \sigma_{i}t + \theta_{wi})$$
 (14b)

Differentiations with respect to time yield the following results for water particle velocities and accelerations

¹¹ Lamb, H., "Hydrodynamics," Sixth Edition, Dover Publications, New York (1945), pp. 363-370, pp. 152-155.

¹²Wehausen, J.V. and E.V. Laitone, "Surface Waves," in "Handbuch der Physik," Vol. 9, Springer Verlag, Berlin (1960), pp. 446-778.

$$\dot{x}_{w} = \sum_{i=1}^{N} \sigma_{i} a_{wi} e^{-k_{i}y} \sin(k_{i}x - \sigma_{i}t + \theta_{wi})$$
 (15a)

$$\dot{y}_{w} = \sum_{i=1}^{N} \sigma_{i} a_{wi} e^{-k_{i}y} \cos(k_{i}x - \sigma_{i}t + \theta_{wi})$$
 (15b)

$$\ddot{x}_{w} = \sum_{i=1}^{N} -\sigma_{i}^{2} a_{wi} e^{-k_{i}y} \cos(k_{i}x - \sigma_{i}t + \theta_{wi})$$
 (16a)

$$\ddot{y}_{w} = \sum_{i=1}^{N} \sigma_{i}^{2} a_{w_{i}} e^{-k_{i} y} \sin(k_{i} x - \sigma_{i} t + \theta_{w_{i}})$$
 (16b)

CHOICE OF COMPONENTS

The computer program allows the user two options for describing an irregular sea. He may specify the values of N, a_{w1} , σ_1 , and θ_{w1} or he may use an energy spectrum $S_S(\sigma)$ to define the wave amplitudes

$$a_{wi} = \sqrt{S_S(\sigma_i) \, \Delta \sigma} \tag{17}$$

where

$$\sigma_i = \sigma_{i-1} + \Delta \sigma$$

$$\Delta \sigma = (\sigma_n - \sigma_0)/N$$

 o_{ii} = upper limit of the significant range of o's

 $\sigma_V = \text{lower limit of the significant range of } \sigma$'s

The program uses the Pierson-Moskowitz energy sea spectrum of the form

$$S_{S}(\sigma) = \frac{A}{\sigma^{5}} e^{-B/\sigma^{4}}$$
 (18)

where A = 0.0081 g² and B = 33.56 h²_{1/3}. Here h_{1/3} is the significant wave height, the average of the one-third highest peak-to-trough heights. As reported by Frank and Salvesen, ¹³ the 11th International Towing Tank Conference (Tokyo) (1966) recommended the spectrum in this form for computations when information is not available on typical sea spectra.

¹³Frank, W. and N. Salvesen, "The Frank Close-Fit Ship-Motion Computer Program," NSRDC Report 3289 (Jun 1970).

Other forms for $S_S(\sigma)$ can, of course, be conveniently programmed. Values for $h_{1/3}$, σ_u , and σ_{ℓ} for State 0 to 9 seas may be found in Table 1 of Reference 13. The program sets the values of θ_{w1} to be evenly spaced from θ_{w1} to $360 \cdot \theta_{w1}$ degrees.

PRESCRIBED SURFACE MOTIONS

The program allows the user either to prescribe the motion at the surface or to describe it by means of differential equations of motion for a surface buoy

If the surface buoy or ship is sufficiently large that its motions are not appreciably affected by the presence of the cable, these motions may be calculated separately and used as input for the present program. Several programs are available to calculate the pitch and heave motion responses of surface ships, for example, the Frank Close-Fit Ship Motion Computer Program.¹³ This approach is also valid for laboratory simulations of cable dynamics where the motions at the upper end of the cable are often prescribed. A third area of application could be a full-scale trial where the motions of the surface ship or platform can be readily measured.

The program considers the prescribed motion of the end of the cable as composed of a series of sinusoidal components in the horizontal and vertical directions

$$x_{s} = \sum_{i=1}^{N} a_{\chi_{i}} \cos(-2\pi f_{i} t + \theta_{si})$$
 (19)

$$y_{s} = \sum_{i=1}^{N} -a_{y_{1}} \sin \left(-2\pi f_{1} t + \theta_{s_{1}}\right)$$
 (20)

where x_s, y_s = horizontal and vertical components of the surface motion, respectively a_{xi}, a_{yi} = amplitudes for the ith component of x_s and y_s , respectively f_i, θ_{si} = frequency and phase angle for the ith component, respectively

If so desired, other forms for the prescribed motion may be conveniently added to the program, e.g., other functions of time such as powers of t or exponentials

SURFACE BUOY EQUATIONS

GENERAL CONSIDERATIONS

It is well known that the added mass and damping coefficients of surface buoys are, in general, functions of the frequency of the oscillation. It is in the time domain, this requires the solution of integrodifferential equations which contain convolution integrals. Alternatively, if the frequency-dependent coefficients can be expressed as simple polynomials of the frequency, the integrodifferential equations may be replaced by a set of higher order differential equations. In either case, the solutions are complex and/or time-consuming in the time domain. Thus, surface buoy motions have usually been solved in the frequency domain. In this approach, the steady-state harmonic response is obtained for each frequency component of the exciting surface waves. The total response to the sum of the individual wave components is then obtained by linear superposition. Experiments have shown that this procedure generally yields satisfactory results for pitch and heave motions of surface ships.

Because of difficulties in solving buoy equations in the time domain, the frequency domain approach has also been used to study the motion of cable-buoy systems. Perhaps the most comprehensive of these studies is the Goodinan et al. computer program¹⁶ which considers four different buoy shapes. In addition to facilitating the solution for general buoy shapes, the frequency domain approach has the additional advantages of immediately giving the steady-state harmonic response (no need to wait for the transient response to die down) and of reducing the computer time required to obtain cable motions.¹⁷ However, the drawbacks to this approach include neglect of all nonlinearities and the assumption that all the dynamic response variables are small compared to their steady-state values. This approach would not be able to predict, for example, the large dynamic snap loads which occur when the cable goes slack.

In view of the above drawbacks and also in view of the existence of the comprehensive frequency-domain computer program described in Goodman et al., 16 it was decided to use a

¹⁴Tick, L.J., "Differential Equations with Frequency-Dependent Coefficients," Journal of Ship Research, Vol. 3, No. 2, pp. 45-46 (Oct 1959).

¹⁵Ogilvie, T.F., "Recent Progress toward the Understanding and Prediction of Ship Motions," Fifth Symposium of Naval Hydrodynamics, Bergen, Norway, pp. 3-128 (Sep. 1964).

¹⁶Goodman, T.R. et al., "Static and Dynamic Analysis of a Moored Buoy System," National Data Buoy Center Report 6113.1 (Apr 1972).

¹⁷Wang, H.T., "A Two-Degree-of-Freedom Model for the Two-Dimensional Dynamic Motions of Suspended Extensible Cable Systems," NSRDC Report 3663 (Oct 1971).

time domain approach in the present study. In order to make this approach leasible, it was important to find classes of buoys which did not have frequency-dependent added mass and damping coefficients. A literature search revealed two such classes: spar buoys 18,19 and small buoys. Spar buoys are buoys with circular cross sections and large draft-to-diameter ratios. H/b. Because of their slenderness, the added inertia terms of these buoys are essentially those for infinite fluid, and the frequency-dependent wave damping coefficients are zero to first order approximation. The other class corresponds to buoys whose typical dimension a is so small compared to the ocean wavelengths λ that the reduced frequency $\bar{\sigma}$ given by

$$\bar{\sigma} = 2\pi \frac{a}{\lambda} \ll 1 \tag{21}$$

is much lest than unity for the range of λ values corresponding to ocean waves of interest.

For surface buoys of sonobuoy systems, whose typical dimension is of the order of 1 ft (0.305 m), the above condition holds for the large majority of sea states. When Equation (21) holds, the wave damping terms go to zero and the added mertia terms for $\overline{v} = 0$ may be used. In this case, the ocean surface behaves essentially as a rigid plane, ²⁰ and for the case of a buoy whose axis of symmetry is aligned with the y-axis, the added mass in surge is equal to the infinite fluid value. The added mass coefficients for pitch and heave must be calculated separately for each shape considered. Since these coefficients have been studied for several cases of oblate and prolate spheroids, 2^{1-24} and also because they represent mathematical shapes which are similar to surface buoys of sonobuoy systems, it was decided to represent the small buoys by prolate and oblate spheroids. Both types of spheroids are characterized by having two of their three axes equal in length. The limiting cases for a prolate spheroid are a

¹⁸Newman, J.N., "The Motions of a Spar Buoy in Regular Waves," David Taylor Model Basin Report 1499 (May 1963).

¹⁹Rudnick, P., "Motion of A Large Spar Buoy in Sea Waves," Journal of Ship Research, Vol. 11, No. 4, pp. 257-267 (Dec 1967).

²⁰Newman, J.N., "Marine Hydrodynamics (Lecture Notes)," M.I.T. Dept. Nav. Arch. and Mar. Ling. (Spring Term 1971).

²¹ Havelock, T., "Waves due to a Floating Sphere Making Periodic Heaving Oscillations," Preceedings of the Royal Society, Vol. 231, Series A, pp. 1-7 (Jul 1955).

²²MacCamy, R.C., "On the Heaving Motion of Cylinders of Shallow Draft," Journal of Ship Research, Vol. 7, No. 3, pp. 34-43 (Dec 1961).

²³Kim, W.D., "On the Forced Oscillations of Shallow-Draft Ships," Journal of Ship Research, Vol. 7, No. 2, pp. 7-18 (Oct. 1963).

²⁴Kim, W.D., "On a Free-Floating Ship in Waves," Journal of Ship Research, Vol. 10, No. 3, pp. 182-191 (Sep. 1966).

long thin cylinder and a sphere, and the limiting cases for an oblate spheroic are a thin circular disk and again a sphere. It can be seen from these limiting cases, that prolate and oblate spheroids can be used to generate a wide range of shapes.

DIFFERENTIAL EQUATIONS OF MOTION

The linearized differential equations for the surge, heave, and pitch motions of a surface buoy or ship freely floating in an inviscid fluid are well known; see, for example, Frank and Salvesen¹³ and Newman. These equations usually contain the inertia forces (including the added hydrodynamic inertia forces), the wave-damping forces, the exciting forces due to the incoming ocean waves, and the restoring forces due to buoyancy. The resulting equations are usually solved in the frequency domain. As mentioned previously, the wave-damping forces may be neglected, to first order, for the two types of buoys considered in the present report. The forces due to viscous drag and cable tension, which are not usually considered in the above studies, are included in the present formulation. The viscous drag forces which are quadratic in the motion velocities of the float, are usually omitted since they make the equations nonlinear and complicate solutions in the frequency domain. The inclusion of these forces poses no problem in a time-domain analysis. The cable forces are, of course, zero for a freely floating buoy.

If the above forces acting on the buoy (shown in Figure 2) are considered and the pitch angle ψ is taken to be small such that

$$\sin \psi \approx \psi \tag{22a}$$

$$\cos \psi \approx 1$$
, (22b)

the three differential equations for the surge ξ , heave ζ , and pitch ψ are as follows

$$\left[A_{\xi\xi} = (m + K_S \rho V)\right] \ddot{\xi} + \left[A_{\xi\psi} = -\mu \int_0^H (y - y_G) k_S(y) S(y) dy\right] \hat{\psi} = FK_x + D_x + T_x + T_{IWx}$$
(23)

$$\left[A_{\eta\eta} = (m + K_{H} \rho V)\right] \zeta = -\rho g S_{w} (\zeta - y_{w}) + F K_{v} + D_{y} + T_{y} - T_{ys}$$
 (24)

$$A_{\xi\psi} \ddot{\xi} + \left\{ A_{\psi\psi} = \left[1 + \rho \int_{0}^{H} (y - y_G)^2 k_S(y) S(y) dy \right] \right\} \ddot{\psi} = \rho g V \overline{BG} \psi + FK_{\psi}$$

$$- \overline{BG} D_x + (-r_y \psi + r_x) T_y - (r_x \psi + r_y) T_x - r_{wy} T_{1Wx}$$
 (25)

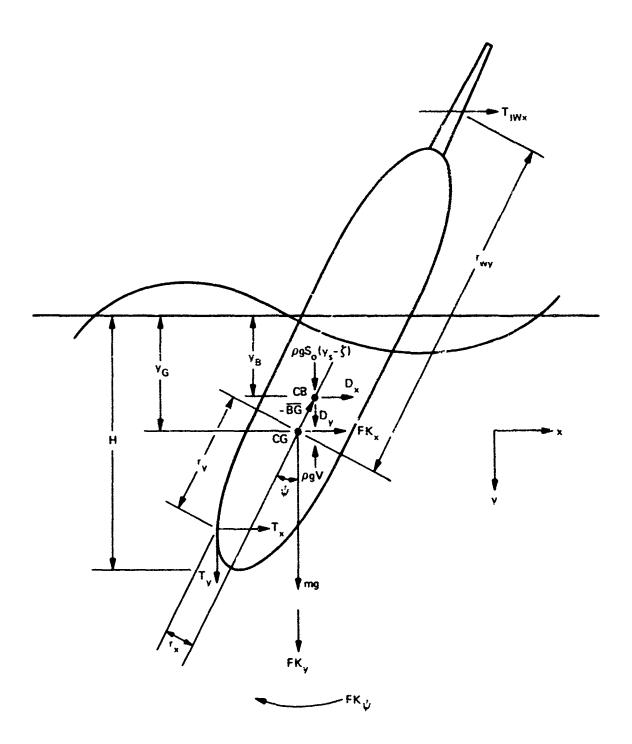


Figure 2 - Definition of Forces Acting on Surface Buoy

 $A_{\xi\xi}, A_{\xi\psi}, A_{\eta\eta}$, and $A_{\psi\psi}$ = inertia coefficients defined above where = mass of the buoy K_S and K_H = added mass coefficients for surge and heave, respectively = submerged volume H = draft = distance of the center of gravity below the undisturbed free surface y_c = local added mass coefficient for surge $k_c(y)$ = local cross-sectional area S(v) = exciting forces due to surface waves in the x- and y-directions. FK_x. FK_y respectively = exciting moment about the center of gravity due to surface waves FK, = viscous drag forces in the x- and y-directions, respectively D_{v}, D_{v} = components of the cable tension in the x- and y-directions, T_{κ}, T_{ν} respectively, at the attachment point to the buoy T_{iwx} = wind loading on the buoy in the x-direction = steady-state component of tension in the v-direction at the Tvs attachment point to the buoy = waterplane area of the buoy S. = vertical displacement of the ocean surface, defined in Equation (14b) $y_{\mathbf{w}}$ I = moment of inertia about the center of gravity $- \beta \overline{G}$ $= y_B - y_G$ = distance of the center of buoyancy below the undisturbed free surface) R = horizontal and vertical distances measured from the center of gravity to the cable attachment point - vertical distance measured from the center of gravity to the center of wy the vand loading force

In Equations (23)-(25), a dot denotes differentiation with respect to time.

The added inertia coefficients, presented below, are all calculated for the case $\psi \equiv 0$. It should be noted that the moment terms on the right-hand side of Equation (25) may be readily changed to account for large values of ψ which negate approximations (22a) and (22b). However, the added inertia terms on the left-hand side of Equations (23) through (25) must be calculated (by pote itial flow methods) for each new value of ψ . For example, for a large value of ψ , the added mass in surge for a small spheroidal buoy is no longer equal to the infinite fluid value since in this case, there will be motion of the fluid perpendicular to the free surface.

Under static conditions, the submerged volume V must support both the weight of the buoy in air (mg) and the vertical component of the steady-state tension (I_{χ_S})

$$\rho gV = mg + T_{VS} \tag{26a}$$

$$V = \frac{m}{\rho} + \frac{T_{ys}}{\rho g}$$
 (26b)

If the input dimensions for the draft and cross-sectional areas of the buoy are such that the submerged volume does not equal the value given in Equation (26b), the program internally multiplies the cross-sectional areas by a constant factor so that the submerged volume becomes exactly equal to the value given by this equation. All of the added inertia and wave-exciting forces given in the following sections are based on this volume.

The following two sections present derivations for the added inertia coefficients K_S , K_H , k_S and the wave-exciting forces FK_K , FK_V , FK_V for the two classes of buoys considered: spar buoys and small spheroidal buoys.

SPAR BUOYS

Because of the slenderness of the spar buoy. Newman¹⁸ shows that its added inertias for surge and pitch are identical to those in infinite fluid. Since the spar buoy has a circular cross section, the surge inertia coefficients k_S and K_S are both equal to 1, leading to the following definitions for the added surge and pitch inertia terms in Equations (23) through (25):

$$K_s = k_s = 1 \tag{27a}$$

$$m + K_S \rho V = m + \rho V \tag{27b}$$

$$\rho \int_{0}^{H} (y - y_G) k_S(y) S(y) dy = \rho \int_{0}^{H} (y - y_G) S(y) dy$$
 (27c)

$$\rho \int_0^H (y - y_G)^2 k_S(y) S(y) dy = \rho \int_0^H (y - y_G)^2 S(y) dy$$
 (27d)

Newman takes the buoy to be sufficiently slender so that the added mass for heave may be neglected. Adee and Bai²⁵ have shown experimentally that for the case of a circular cylinder, it is more accurate to add a term corresponding to one-half the added mass of a circular disk (with the same diameter as that of the cylinder) heaving in infinite fluid.

²⁵ Adee, B.H. and K.J. Bai, "Experimental Studies of the Behavior of Spar Type Stable Platforms in Waves," University of California (Berkeley) Report NA-70-4 (Jul 1970).

This correction term has been incorporated into the present formulation by takir z the radius of the disk to be the mean radius of the spar buoy \bar{r} , defined by

$$\pi \bar{r}^2 H = V, \ \bar{r} = \sqrt{\frac{V}{\pi H}}$$

Since one-half of the added inertia of a circular disk heaving in infinite fluid¹¹ is $(4/3) \rho \bar{r}^3$, the following expression is obtained for K_H in Equation (24)

$$K_{\rm H} = \frac{4}{3} \frac{1}{V} \bar{r}^3 = \frac{4}{3} \frac{1}{V} \left(\frac{V}{\pi H} \right)^{3/2}$$
 (28)

Newman shows that the wave-exciting forces are simply the Froude-Krylev forces, which may be obtained by integrating the pressure field generated by the ocean waves around the contour of the buoy. The following expressions are obtained for FK_x and FK_{tt}

$$FK_{x} = -2 \sum_{i=1}^{N} \left[\sigma_{i}^{2} a_{wi} \cos(k_{i}x - \sigma_{i}t + \theta_{wi}) Q_{o}(k_{i}) \right]$$
 (29)

$$FK_{\psi} = 2 \sum_{i=1}^{N} \left[\sigma_{i}^{2} a_{wi} \cos(k_{i}x - \sigma_{i}t + \theta_{wi}) Q_{1}(k_{i}) \right]$$
 (30)

where

$$Q_o(k_i) = \rho \int_0^H e^{-k_i y} S(y) dy$$

$$Q_1(k_i) = \rho \int_0^H e^{-k_i y} (y - y_G) S(y) dy$$

For the heave motion, the Froude-Krylov force given by Newman has been modified to account for the additional heave added-mass term given in Equation (28). This modification has been so made that in the limiting case of a very small spar buoy (which would follow the motions of the waves in the absence of cable forces), the wave-exciting term in Equation (24) would be exactly equal to the inertia term on the left-hand side of the equation. The resulting force has the from

$$FK_{y} = \left[1 + \frac{4}{3} \rho \left(\frac{V}{\pi H}\right)^{3/2}\right] \sum_{i=1}^{N} \left[\sigma_{i}^{2} a_{wi} \sin(k_{i}x - \sigma_{i}t + \theta_{wi}) Q_{o}(k_{i})\right]$$
(31)

The expression given by Newman does not contain the correction term $(4/3) \rho (V/\pi H)^{3/2}$.

SMALL SPHEROIDAL BUOYS

As mentioned previously, for the limiting case of zero reduced frequency where the free surface behaves as a rigid plane, the coefficient for the surge added mass K_S is equal to the infinite fluid value. These coefficients may be calculated by the formulas and tables given in Lamb.¹¹

The added inertia coefficients for heave, pitch, and coupled pitch-surge motions which have components normal to the free surface are not the same as the coefficients for infinite fluid. Instead, they must be calculated separately to incorporate the rigid free-surface condition. Inspection of Equations (23) and (25) shows that the added inertia terms for pitch and coupled pitch-surge motions may be defined as follows.

$$\int_{0}^{H} (y - y_G)^2 k_S(y) S(y) dy = \int_{0}^{H} (y^2 - 2yy_G + y_G^2) k_S(y) S(y) dy$$

$$= \mu_{55} r_w^2 V - 2y_G (-\mu_{15}) r_w V + y_G^* K_S V$$
(32a)

$$-\int_{0}^{H} (y - y_G) k_S(y) S(y) dy = \mu_{1S} r_W V + y_G K_S V$$
 (32b)

where r_w is the maximum radius of the buoy and is used to render the coefficients μ_{55} and μ_{15} dimensionless.

Thus, a calculation of μ_{55} , μ_{15} , and K_H , along with the values of K_S for infinite fluid as given in Lamb, completely determines all the added inertia terms in Equations (23) through (25). Bai²⁶ has used a finite element approach to calculate coefficients μ_{55} , μ_{15} , and K_H for spheroids with draft to maximum radius ratios (H/r_w) ranging from 0.1 to 10. His results agree well with previous results²¹⁻²⁴ for corresponding cases at the zero reduced frequency limit.

Principally for the sake of programming ease, the present program considers only the results for the case where the maximum radius lies at the free surface. Consideration of other radii at the free surface would introduce additional parameters to a description of the submerged buoy. In addition, it is expected that in most cases the maximum radius will be close to the free surface because most of the volume (and hence buoyancy capability) of the spheroid is concentrated in the region around the maximum radius. Results for several cases where the waterline does not occur at the maximum radius are presented in Bai. 26

²⁶Bai, K.J., "The Zero-Frequency Hydrodynamic Coefficients of Vertical Axisymmetric Bodies at a Free Surface," Journal of Hydronautics (Jan 1977).

In the range $0.1 \le H/r_w \le 10$., the coefficients μ_{15} , μ_{55} , K_H , and K_S are obtained by linearly interpolating between the values shown in Table 1.

TABLE 1 - VALUES OF ADDED INERTIA COEFFICIENTS FOR SPHEROIDAL BUOYS AT ZERO REDUCED FREQUENCY

H/rw	<u>K</u> s	<u>к</u> н	μ ₅₅	μ_{15}
0.1	0.074	12.84	1.27	0.3
0.2	9.143	5.84	0.5\$	0.264
0.3	0.2%→	3.672	0.312	0.237
0.5	0.31	2.005	0.117	0.177
0.7	0.397	1.323	0.0358	0.11
0.9	0.469	0.96	0.0038	0.0039
1.0	0.50	0.836	0.	0.
1.5	0.622	0.484	0.0731	-0 191
2.	0.704	0.323	0.272	-0.391
3.	0.804	0.18	0.993	-0.788
5 .	0.894	0.082	3.65	-1.565
7.	0.933	0.049	8.20	-2.38
10.	0.96	0.028	20.0	-3.75

The values of K_S are those given by Lamb, ¹¹ and the values of μ_{15} , μ_{55} , and K_H are the values given by Bai, ²⁶ subject to two modifications at $H/r_w = 10.0$. The values of μ_{15} and μ_{55} for $H/r_w = 10$, respectively 20.0 and -3.75, correspond to those obtained by simply using strip theory with $k_S(y) = 1$ for $0 \le y \le H$. The corresponding values of μ_{15} and μ_{55} obtained by Bai are respectively 17.84 and -3.53. These modifications were made principally for the sake of providing continuity with the approximation used in the range $H/r_w > 10.0$, where the buoy is treated essentially as a spar buoy. There are two reasons for the differences between the strip theory and theoretical finite element calculations. ²⁶ First, the strip theory approach neglects the flow around the lower end of the buoy, where $k_S(y) \le 1$. Second, the finite element representation, where only the nodes of the elements are on the surface of the buoy, effectively models a smaller buoy. Both of these effects serve to make the strip theory values higher than the corresponding finite element results.

Wave-Exciting Forces

The exciting forces FK_x and FK_y in Equations (23) and (24) were written in a form which assumes that the buoy follows the wave motion in the absence of cable forces and

coupling between pitch and surge motions. This is the case for buoys with dimensions which are small compared to the lengths of the exciting waves. The resulting equations take the form

$$FK_{x} = (1 + K_{S}) \rho V \ddot{x}_{w} = -(1 + K_{S}) \rho V \sum_{i=1}^{N} \left[\sigma_{i}^{2} a_{w_{i}} \cos(k_{i} x - \sigma_{i} t + \theta_{w_{i}}) \right]$$
 (33)

$$FK_{y} = (1 + K_{H}) \rho V \ddot{y}_{w} = (1 + K_{H}) \rho V \sum_{i=1}^{N} \left[\sigma_{i}^{2} a_{wi} \sin(k_{i}x - \sigma_{i}t + \theta_{wi}) \right]$$
(34)

The term e^{-k_1y} which appears in Equations (16a) and (16b) for \ddot{x}_w and \ddot{y}_w has been omitted in the above equations since it is ≈ 1 under the assumption of small reduced frequency. Equation (21).

The pitch-exciting moment is computed by noting that because of the symmetry of the buoy about the vertical axis, only the horizontal wave motions make a contribution to pitch, resulting in

$$FK_{\psi} = -\rho \int_{0}^{H} (y - y_{G}) \ddot{x}_{w} [1 + k_{S}(y)] S(y) dy$$

$$= \rho \left(-\overline{BG} V + \mu_{1S} r_{w} V + y_{G} K_{S} V \right) \ddot{x}_{w}$$

$$= -\rho V \left(-\overline{BG} + \mu_{1S} r_{w} + y_{G} K_{S} \right) \sum_{i=1}^{N} \left[\sigma_{i}^{2} a_{w_{i}} \cos (k_{i} x - \sigma_{i} t + \theta_{w_{i}}) \right]$$
(35)

Again, under the assumption of small reduced frequency, the term $e^{-k_1 y}$ has been set equal to 1 in the expression for \tilde{x}_w .

DYNAMIC CABLE EQUATIONS

GENERAL CONSIDERATIONS

As mentioned previously, equations for the present study are solved in the time domain. Previous cable studies have considered two major approaches in the time domain: the method of characteristics and the finite element method. The method of characteristics is an elegant method which reduces the original set of partial differential equations to a set of ordinary differential equations which are integrated along characteristic lines or wavefronts. This method furnishes valuable insight into the various modes of cable motion but solution times are typically very large.²

The finite element method seeks to represent the actual cable system by a series of segments and nodes. The original set of partial differential equations is then reduced to a set of ordinary differential equations of motion for the nodes. This method facilitates the modeling of nonuniform properties along the cable as well as the presence of intermediate bodies. The method is also quite flexible in that the number and location of nodes is left to the judgment of the user. In arriving at his selection of nodes, he may consider such factors as the type and accuracy of the dynamic information desired, the amount of computer time available, the complexity of the cable system, and the spatial variation of the environmental velocity profiles.

Principally for reasons of generality and flexibility, the finite element approach was used to model the cable. Straight elements are used in the present formulation. Webster²⁷ studied the use of higher order curved elements to model cable shape and concluded that the first order straight element appears to be the most cost-effective. In particular, he showed that one second-order quadratic element would have to be as accurate as at least eight straight elements before its use would be economical.

Before deciding on the final formulation, a number of preliminary approaches for obtaining the differential equations of motion for the nodes were explored. Two approaches in particular were considered in some detail.

PRELIMINARY APPROACHES

In one approach, the equations were formulated in a coordinate system aligned with the cable segment. This is the approach used by Rupe and Thresher²⁸ to obtain the dynamic motions of an inextensible, uniform cable. For the present case of an extensible cable, the two unknowns are the inclination and stretch of each segment. This is the most natural way of describing the configuration of a segment. In addition, certain cable forces such as tension, added inertia, and drag forces are most conveniently expressed in directions normal and tangential to a cable segment. However, in the presence of intermediate bodies along the cable, for which the inertia and drag are most conveniently expressed in the spatial x- and y-directions, the resulting equations are greatly complicated by the transformation required to express the body forces in the cable coordinate system. The cable system considered by Rupe and Thresher is free of intermediate bodies.

²⁷Webster, R.L., "An Application of the Finite Element Method to the Fetermination of Nonlinear Static and Dynamic Responses of Underwater Cable Structures," General Electric Report R76EMH2 (Jan 1976).

²⁸Rupe, R.C. and Thresher, R.W., "The Anchor-Last Deployment Problem for Inextensible Mooring Lines," ASME Paper 74-WA/OcT-5 (Dec 1974).

Wang²⁹ has shown that relatively few segments are required to accurately describe the overall steady-state configuration of a cable; these results suggested a second novel approach. The cable was conventionally divided into a number of straight segments and two differential equations in the x- and y-directions were written for the nodes at the ends of the segments. However, each straight segment was subdivided into a number of intermediate nodes, as shown in Figure 3.

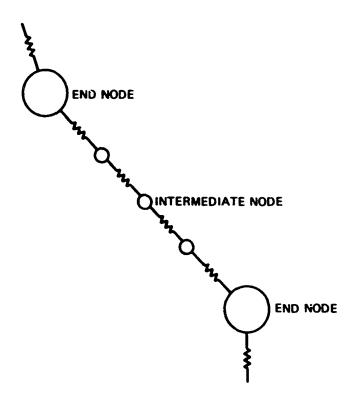


Figure 3 - Finite Element with Intermediate Nodes

Since these intermediate nodes were forced to move along the straight cable segment, only one differential equation was needed to describe their longitudinal motion. The principal intention of this approach was to have the end nodes describe the overall cable configuration and the intermediate nodes describe the variation of tension along a cable segment. There was not sufficient time in the present study to fully explore this approach. However, it was found that a certain amount of bookkeeping was required in the program to differentiate between the "end" and "intermediate" nodes. Also, although this approach reduces the total number of differential equations from that required by more conventional approaches, the

²⁹Wang, H.T., "Determination of the Accuracy of Segmented Representations of Cable Shape," Journal of Engineering for Industry, Vol. 97, No. 2, Series B, pp. 472-478 (May 1975).

integration time step still depends on the distance between intermediate nodes and the elastic modulus of the cable. For short distances and nearly inextensible cables, the size of the integration steps becomes very small, which increases computer time.

FINAL FORMULATION

In view of the above, selection of the final finite element model was as shown in Figure 4. The continuous cable is divided into a number of massless straight elastic segments. The inertia, weight, and drag forces acting on each cable segment are equally divided between the two nodes at the ends of the segment.

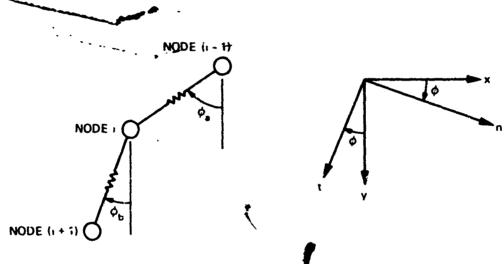


Figure 4 - Final Finite Element Model

Two second-order differential equations of motion are written in the spatial x- and y-directions for each node i, as follows

$$J_{x_{i}}\ddot{x}_{i} + K_{i}\ddot{y}_{i} = F_{x_{i}} - T_{x_{i}} + D_{nx_{i}} + D_{tx_{i}} + D_{Bx_{i}}$$
 (36)

$$K_i \ddot{x}_i + J_{y_i} \ddot{y}_i = F_{y_i} = T_{y_1} + D_{ny_i} + D_{ty_1} + D_{By_i} + W_{C_i} + W_{B_i}$$
 (37)

 $i = 1, 2, \ldots, M$

where J_x , J_y , K = inertia coefficients defined below in Equation (43)

x, y = subscripts denoting the x- and y-directions, respectively

F = sum of the tension, drag, and gravity forces acting on the node

T = tension in the cable due to stretch and internal damping

D_n = normal drag force acting on the cable

D_t = tangential drag force acting on the cable

 D_{μ} = drag force acting on the intermediate body

W_c = weight of the cable in fluid

 W_{p} = weight of the intermediate body in fluid

M = total number of nodes

Detailed definitions of the cable forces are now given, and the drag and mertia forces for the intermediate bodies are defined in the next major section.

It is most convenient to solve the set of 2M differential equations (36) and (37) with \vec{x}_1 and \hat{y}_1 uncoupled, as follows

$$\ddot{x}_i \approx (J_{y_i} F_{x_i} - K_i F_{y_i})/(J_{x_i} J_{y_i} - K_i^2)$$
 (38a)

$$\ddot{y}_i = (J_{x_i} F_{y_i} - K_i F_{x_i})/(J_{x_i} J_{y_i} - K_i^2)$$
 (33b)

DEFINITION OF CABLE FORCES

Inertia Forces

Each cable segment is taken to be a long thin cylinder for which fluid inertia is added only for acceleration normal to the segment. Thus, in a coordinate system aligned with the segment, the inertia force \vec{F}_{IC} for the cable segment is simply given by

$$\vec{F}_{IC} = (\mu \ell_0 + \alpha \rho A \ell_0) \vec{a}_n + \mu \vec{e}_0 \vec{a}_t$$
 (39)

where

 μ = mass per unit length of the cable

 ℓ_0 = reference length of the cable segment

 α = added mass coefficient = 1.0 for a round cable

A = cross-sectional area of the cable

 $\vec{a}_n, \vec{a}_t =$ accelerations respectively normal and tangential to the cable segment

In this equation, an arrow denotes a vector.

In the fixed x- and y-directions, for which the differential equations are written, the inertia coefficients are not constants but rather are functions of cable inclination ϕ . For the coordinate systems shown in Figure 4, a_n and a_t are related to \hat{x} and \hat{y} , respectively the accelerations in the x- and y-directions, by

$$a_n = \ddot{x} \cos \phi - \ddot{y} \sin \phi \tag{40a}$$

$$a_t = -\ddot{x} \sin \phi + \ddot{y} \cos \phi \tag{40b}$$

The x- and y-components of a_n and a_t are, in turn, given by

$$a_{nx} = a_n \cos \phi = \ddot{x} \cos^2 \phi + \ddot{y} \sin \phi \cos \phi \tag{41a}$$

$$a_{nv} = a_n \sin \phi = \ddot{x} \sin \phi \cos \phi + \ddot{y} \sin^2 \phi \tag{41b}$$

$$a_{tx} = -a_t \sin \phi = \ddot{x} \sin^2 \phi - \ddot{y} \sin \phi \cos \phi \qquad (41c)$$

$$a_{tv} = a_t \cos \phi = -\ddot{x} \sin \phi \cos \phi + \ddot{y} \cos^2 \phi$$
 (41d)

The inertia force for the cable se, ment in the spatial coordinates x and y takes the form

$$\vec{F}_{IC} = [(\mu \ell_o + \alpha \rho A \ell_o) a_{nx} + \mu \ell_o a_{tx}] \vec{i}$$

$$+ \{(\mu \ell_o + \alpha \rho A \ell_o) a_{ny} + \mu \ell_o a_{ty}\} \vec{j}$$
(42)

where \vec{i} , \vec{j} are unit vectors in the x- and y-directions, respectively.

If one-half of the inertia force of the cable segments above and below the node are summed and the possible presence of an intermediate body at the node is taken into account. Equations (39) to (42) yield the following equation for the total inertia force \overrightarrow{F}_1 at the node

$$\vec{F}_{1} = \left[\left(\frac{\mu_{a} \ell_{oa} + \mu_{b} \ell_{ob}}{2} + \frac{\alpha \rho A_{a} \ell_{oa}}{2} \cos^{2} \phi_{a} + \frac{\alpha_{b} \rho A_{b} \ell_{ob}}{2} \cos^{2} \phi_{b} + M_{BVX} \right) \ddot{x}$$

$$+ \left(\frac{-\alpha \rho A_{a} \ell_{oa}}{2} \sin \phi_{a} \cos \phi_{a} - \frac{\alpha \rho A_{b} \ell_{ob}}{2} \sin \phi_{b} \cos \phi_{b} \right) \ddot{y} \right] \vec{i}$$

$$+ \left[\left(\frac{-\alpha \rho A_{a} \ell_{oa}}{2} \sin \phi_{a} \cos \phi_{a} - \frac{\alpha \rho A_{b} \ell_{ob}}{2} \sin \phi_{b} \cos \phi_{b} \right) \ddot{x}$$

$$+ \left(\frac{\mu_{a} \ell_{oa} + \mu_{b} \ell_{ob}}{2} + \alpha \rho A_{a} \ell_{oa} \sin^{2} \phi_{a} + \alpha \rho A_{b} \ell_{ob} \sin^{2} \phi_{b} + M_{BVY} \right) \ddot{y} \right] \vec{j}$$

$$= (J_{x}\ddot{x} + K\ddot{y}) \vec{i} + (K\ddot{x} + J_{y}\ddot{y}) \vec{j}$$

$$(43)$$

where subscripts a and b respectively denote the cable segments above and below the node and M_{BVX} and M_{BVY} are respectively the virtual mass, the mass plus the added mass, of the intermediate body for motions in the x- and y-directions.

Tension Force

For an extensible cable segment, the tension force T depends on the strain ϵ and the strain rate $\dot{\epsilon}$

$$T = T(\epsilon, \dot{\epsilon}) \tag{44}$$

where

$$\epsilon = \frac{\Delta \ell}{\ell_0} = \frac{\ell}{\ell_0} - 1 = \frac{\sqrt{(\chi_\ell - \chi_u)^2 + (\chi_\ell - \chi_u)^2}}{\ell_0} - \frac{1}{\ell_0}$$
(45)

$$\dot{\epsilon} = \frac{d\epsilon}{dt} = \frac{(\dot{x}_{\ell} - \dot{x}_{u})(x_{\ell} - x_{u}) + (\dot{y}_{\ell} - \dot{y}_{u})(y_{\ell} - y_{u})}{\xi \ell_{o}}$$
(46)

Here C is the stretched length of the cable segment and the subscripts C and u respectively refer to the lower and upper ends of the cable segment.

Several relationships have been proposed in the literature for the form of this dependence. In the present study, the following relatively simple yet general relationship is used

$$T = T_0 + C_1 e^{C_2} + C_1 \dot{e}$$
 (47)

where C_t is the internal damping coefficient. Note that the above dynamic tension differs from the static tension, Equation (8) by the addition of the linear internal structural damping term $C_t \hat{\epsilon}$. This model, commonly referred to as the Voigt model, has been used in previous cable studies, for example, by Huffman^{30} and by Goeller and Laura. Goeller and Laura show experimentally that this model adequately describes internal damping for hylon ropes except for frequencies significantly higher than the resonance frequency of the cable. Other more complex forms for the internal structural damping, such as those proposed by Reid. Can be conveniently incorporated into the program.

Since the cable segments above and below the node act on it, the following two expressions are obtained for T_y and T_y (respectively the x_0 and y-components of the resultant forces due to cable tension).

$$T_x = T_{ax} + T_{bx} = T_a \sin \phi_a - T_b \sin \phi_b \tag{48a}$$

$$T_y = T_{ay} + T_{by} = -T_a \cos \phi_a + T_b \cos \phi_b$$
 (48h)

³⁰Hillman, R.R., "The Dynamical Behavior of Extensible Cable in a Uniform Flow Field (An Investigation of the Towed Vehicle Problem)." Ph.D. Thesis, Purdue University (Jan 1969)

³¹G eller, J.F. and P.A. Laura, "Analytical and Experimental Study of the Dynamic Response of Cable Systems," The Catholic University of America, Department of Mechanical Engineering, Themis Program 893, Report 70-3 (Apr. 1970).

³²Reid, R.O., "Dynamics of Deep-Sea Mooring Lines," Texas A&M University, Department of Oceano-graphy, ACM Project 204, Reference 68-HF (Jul 1968),

Drag Forces

As shown previously in Equations (6) and (7), the static normal and tangential cable drags are taken to be respectively proportional to the squares of the normal and tangential components of the current velocity \vec{c} . In the dynamic case, the fluid velocity relative to the cable must include the ocean wave particle velocities \hat{x}_w and \hat{y}_w as well as the velocities \hat{x} and \hat{y} of the cable. The resultant expressions v_{rn} and v_{rt} for the relative velocities normal and tangential to the cable then take the form

$$\mathbf{v}_{rn} = (\mathbf{c} + \dot{\mathbf{x}}_{\mathbf{w}} - \dot{\mathbf{x}})\cos\phi + (\dot{\mathbf{y}}_{\mathbf{w}} - \dot{\mathbf{y}})\sin\phi = \dot{\mathbf{x}}_{r}\cos\phi + \dot{\mathbf{y}}_{r}\sin\phi \tag{49a}$$

$$v_{rt} = -(c + \dot{x}_w - \dot{x}) \sin \phi + (\dot{y}_w - \dot{y}) \cos \phi = -\dot{x}_r \sin \phi + \dot{y}_r \cos \phi$$
 (49b)

where $\dot{x}_r = c + \dot{x}_w - \dot{x}$ and $\dot{y}_r = \dot{y}_w - \dot{y}$.

If one-half of the drag forces acting on the cable segments above and below the node are summed, the following equations are obtained for the resultant forces D_{nx} , D_{ny} , D_{tx} , and D_{ty} acting on the node

$$D_{nx} = \frac{1}{2} D_{na} \cos \phi_a + \frac{1}{2} D_{nb} \cos \phi_b$$
 (50a)

$$D_{ny} = \frac{1}{2} D_{na} \sin \phi_a + \frac{1}{2} D_{nb} \sin \phi_b$$
 (50b)

$$D_{tx} = -\frac{1}{2}D_{ta}\sin\phi_a - \frac{1}{2}D_{tb}\sin\phi_b$$
 (50c)

$$D_{ty} = \frac{1}{2} D_{ta} \cos \phi_a + \frac{1}{2} D_{tb} \cos \phi_b$$
 (50d)

where

$$\begin{split} &D_{n(a,b)} = \frac{1}{2} \rho \, C_{D(a,b)} \, d_{(a,b)} \, \ell_{o(a,b)} \, v_{m(a,b)} \, |v_{rn(a,b)}| \\ &D_{t(a,b)} = \frac{1}{2} \rho \, C_{T(a,b)} \, d_{(a,b)} \, \ell_{o(a,b)} \, v_{rt(a,b)} \, |v_{rt(a,b)}| \\ &v_{rn(a,b)} = \dot{x}_r \, \cos \phi_{(a,b)} + \dot{y}_r \, \sin \phi_{(a,b)} \\ &v_{rt(a,b)} = -\dot{x}_r \, \sin \phi_{(a,b)} + \dot{y}_r \, \cos \phi_{(a,b)} \end{split}$$

INTERMEDIATE BODIES

The magnitude and direction of the added inertia and drag forces for an arbitrary body depend, in general, in a complex manner on body shape and orientation relative to the flow. The modeling of intermediate bodies in previous cable studies has ranged from the very simple to the very complex. At one extreme, the presence of intermediate bodies has been neglected altogether, leading to a cable-only system. At the other extreme, some studies have paid very careful attention to a particular body, often the lower body of the system, and approximated the rest of the cable system in a simple manner.

After careful review of previous studies, formulations were selected for the present study which although relatively simple, can model most bodies of interest for sonobuoy systems. They are also applicable to other cable systems where the bodies are relatively small and/or conform to the shape limitations given below.

CONSTANT COEFFICIENTS

Inertia Forces

The inertia forces in the x- and y-directions are expressed as the virtual mass times the acceleration in these directions, where the virtual mass is the sum of the mass of the body M_B and the constant infinite fluid added mass. Thus, the virtual masses M_{BVX} and M_{BVY} which appear in Equation (43) are given by

$$\mathbf{M}_{\mathbf{R}\mathbf{V}_{\mathbf{X}}} = \mathbf{M}_{\mathbf{R}} + \mathbf{K}_{\mathbf{Y}} \rho \, \mathbf{V}_{\mathbf{R}} \tag{51a}$$

$$M_{BVy} = M_B + K_y \rho V_B \tag{51b}$$

where K_x , K_y are respectively the added mass coefficients for motions in the x- and y-directions and V_B is the reference volume, usually the volume of the body.

Drag Forces

Two formulations are used to describe the drag forces. In one, the drag components D_{Bx} and D_{By} are taken to have the form

$$D_{Bx} = \frac{1}{2} \rho C_{DAx} v_r \dot{x}_r \tag{52a}$$

$$D_{By} = \frac{1}{2} \rho C_{DAy} v_r \dot{y}_r$$
 (52b)

where C_{DAX} , C_{DAY} are respectively the drag areas in the x- and y-directions and $v_r = \sqrt{\dot{x}_r^2 + \dot{y}_r^2}$ is the resultant fluid velocity relative to the body. This approach has been used by Walton and Polachek³³ and is exact for the case of a sphere. In this case, where $C_{DAX} = C_{DAY} = C_{DA}$, the resultant drag D_B is parallel to the resultant fluid velocity v_r

$$\vec{D}_{B} = D_{Bx}\vec{i} + D_{By}\vec{j} = \frac{1}{2}\rho C_{DA} v_{r}(\dot{x}_{r}\vec{i} + \dot{y}_{r}\vec{j}) = \frac{1}{2}\rho C_{DA} v_{r}\vec{v}_{r}$$

This approach may also be used to approximate the drag for other blunt shapes for which the drag areas for different flow directions do not differ greatly, e.g., near-cubes or circular cylinders with length to diameter ratios of approximately 1.

In the second approach, D_{BX} and D_{BY} are taken as proportional to the squares of the components of the relative fluid velocities in these respective girections

$$D_{Bx} = \frac{1}{2} \rho C_{DAx} \dot{x}_r |\dot{x}_r|$$
 (53a)

$$D_{By} = \frac{1}{2} \rho C_{DAy} \dot{y}_r |\dot{y}_r|$$
 (53b)

This is a good approximation for long cylinders or thin disks with axes parallel to the x- or y-directions. In these cases, where there is a large difference between the drag areas, the drag in one direction is essentially pressure drag whereas the much smaller drag in the other direction is essentially due to fluid friction.

In the present program, the choice of whether to use Equation (52) or (53) to compute the drag is determined by the value of the ratio $C_{\rm DAx}/C_{\rm DAy}$. It was somewhat arbitrarily decided that in the range

$$0.5 \leqslant C_{\text{DAx}}/C_{\text{DAy}} \leqslant 2.0 \tag{54}$$

Equations (52a) and (52b) would be used to compute D_{Bx} and D_{By} . Outside this range, Equations (53a) and (53b) are used to compute D_{Bx} and D_{By} .

VARIABLE COEFFICIENTS FOR CIRCULAR DISK

When a body is executing dynamic oscillations such that it periodically traverses its own viscous wake, the added mass and drag coefficients are more correctly expressed as functions

³³Walton, T.S. and H. Polachek, "Calculation of Transient Motion of Submerged Cables," Mathematical Tables and Other Aids to Computation, Vol. 14, No. 69, pp. 27-46 (Jan 1960).

of the dynamic motion. Drag coefficients are usually obtained for steady flow and added mass coefficients are often computed for an inviscid fluid. The reviews by Wiegel³⁴ and Holler³⁵ show that nearly all of the measurements of the dynamic coefficients for various bodies have been conducted for dynamic oscillation in one direction only and in the absence of any steady-state fluid velocity. Thus, the results from these studies should be used with caution in the present case where the cable system will generally undergo dynamic motions in both directions in the presence of a steady-state current profile.

For the particular case of a circular disk, which is commonly used in sonobuoy systems to damp out the motions of the lower acoustic units, the user may either employ the constant coefficient approach (described previously) or have the program internally compute the dynamic added mass and drag coefficients for the direction normal to the disk plane, as based on the experimental relationships given by Holler.³⁵ The coefficients are related to the dynamic motions as follows³⁵

$$C_{DAn} = \frac{\pi d_B^2}{4} \frac{2.2}{\gamma}$$
 (55a)

$$M_{RVn} = M_R + 1.2 \gamma \rho d_R^2 = M_R + M_A$$
 (55b)

$$\gamma = \sqrt{\beta} = \sqrt{\frac{\hat{n}^2}{\hat{n}d_B}} \text{ for } 0.077 \le \beta \le 3.84$$

$$= \sqrt{3.84} = 1.96 \text{ for } \beta > 3.84$$

$$= \sqrt{0.077} = 0.278 \text{ for } \beta < 0.077$$

where

n = direction normal to the disk plane, either x or y

 M_{\star} = added mass

 \vec{n} , \vec{n} = the velocity and acceleration, respectively, of the disk in this direction, either (\dot{x}, \ddot{x}) or (\dot{y}, \ddot{y})

The above formulas show how the drag area and the added mass vary with β , which is a measure of the ratio of relative magnitudes of the velocity and acceleration. At low values of β , where the acceleration is much higher than the velocity (e.g., during the initial instants of

³⁴Wiegel, R.L., "Oceanographical Engineering," Prentice Hall, Inc., Englewood Cliffs, N.J., (1964), Chapter 11.

³⁵Holler, R.A., "Hydrodynamic Effects of Harmonic Acceleration," Naval Air Development Center Report AE-7120 (Jan 1972).

a body starting from rest), the formulas show that C_{DAn} has a high value and that the added mass is given by $M_A = \rho \, d_B^{\ 3}/3$, the potential flow result. At the other extreme of high values of β , where the velocity is much higher than the acceleration, the added mass has a high value and the drag area is given by $C_{DAn} = 1.12 \, \pi \, d_B^{\ 2}/4$, the steady-state value.

The added mass and drag in the direction tangent to the disk plane, which are much smaller, are computed by the constant coefficient approach outlined previously.

DESCRIPTION OF COMPUTER PROGRAM

Program CABUOY consists of a main program and six subroutines.

MAIN PROGRAM

The main program accepts input data for the cable system, surface waves, current profile, and the initial conditions for the dynamic calculations. If a surface buoy is present, input data are read in by Subroutine BUOY, described below. Data may be entered in either English or metric units. A detailed description of input instructions is given later.

The program is currently written to accept up to 50 cable segments and 49 intermediate bodies. This number can be conveniently increased by changing a few DIMENSION and COMMON statements, but it should be noted that dynamic calculations for more than 50 nodes are likely to require prohibitively large amounts of computer time.

The main program prints out the input data and then calls on various subroutines to calculate the ocean wave spectrum, certain constants for the surface buoy, the steady-state configuration of the cable system, and finally the dynamic motions of the system at prescribed time intervals. The output from the steady-state and dynamic calculations are also printed by this program.

SUBROUTINE STAT

This subroutine defines the five steady-state differential equations, (1) to (5), for the cable.

SUBROUTINE DYNA

This subroutine defines the dynamic differential equations, (36) and (37), for each of the M nodes. For cases where a buoy is present, this subroutine also defines the three differential equations of motion for the surface buoy, namely (23) to (25).

SUBROUTINE CUR

This subroutine furnishes the steady-state current profile relative to the cable system. For a free-floating cable system, this would be the actual current profile minus the drift velocity of the cable system. For a given value of the vertical distance y, this subroutine linearly interpolates between the input velocities which are read in as a function of y. For cases where the given value of y is greater (less) than the largest (smallest) value of y which is read in, the subroutine takes the velocity to be the value at the largest (smallest) algebraic value of y which is read in.

SUBROUTINE SPECT

This subroutine is employed when the user wishes the program to internally generate the surface wave components. In this case, the subroutine defines the amplitudes of the surface wave components by using the Pierson-Moskowitz energy sea spectrum. Equation (18). Provision is left at the end of the subroutine for implementing other forms for the sea spectrum.

SUBROUTINE BUOY

This subroutine is used when a surface buoy is present. After reading input data for the surface buoy, the subroutine calculates the various buoy geometrical and added inertia coefficients which appear in Equations (27) to (35). It concludes by calculating the steady-state pitch angle of the buoy, in the absence of any dynamic excitation due to surface waves.

SUBROUTINE ITERA

This subroutine is used for boundary value cases when iteration schemes are required to obtain the steady-state configuration of the cable system. It contains iteration schemes which are applicable for free-floating cable systems and a cable of given length moored in a given ocean depth. Provision is made at the end of the subroutine for implementing iteration schemes for other applications.

SUBROUTINE KUTMER

This subroutine uses the Kutta-Merson method to numerically integrate the steady-state differential equations defined in Subroutine STAT and the dynamic differential equations of

motion defined in Subroutine DYNA. The subroutine automatically reduces the integration step size until specified error criteria are met.

INPUT INSTRUCTIONS

READ STATEMENTS

Input data are entered into the program by means of the following READ statements contained in Program MAIN and Subroutine BUOY. These statements are given numbers simply for identification purposes.

MAIN Program

READ (5,1) NO	CASES	Card	1
DO 1000 MC=1.	NCASES		
READ (5,301)	TITLE	Card	2
READ (5.1) NS	M, NSW, NCAB, NCUR, ITER, MTRC	Card	3
READ (5,2) (FS	SM(K), K = 1, NSM	Card	4
READ (5,2) (A)	XSM(K), K=1, NSM)	Card	5
READ (5,2) (A)	YSM(K), K = 1, NSM)	Card	6
READ (5,2) (FI	DSM(K), K = 1, NSM)	Card	7
READ (5,2) (AS	SW(K), K = 1, NSW)	Card	8
KEAD (5,2) (FF	RSW(K), K = 1, NSW)	Card	9
READ (5,2) (FI	DSW(K), K = 1, NSW)	Card	10
READ (5,2) RH	IO, SUBM, TWX, TIY, CDASX, AMC, AFAC, TMIN	Card	11
READ (5,2) TIN	NV1, DT1, TOTT, DT2, DIR, TBH, TBYMX	Card	12
READ (5,3) (FI	LC(K), K = 1, NCAB)	Card	13
READ (5.2) (DO	CI(K), K = 1, NCAB	Card	14
READ (5,2) (CI	DN (K), K = 1, NCAB)	Card	15
READ (5,2) (CI	DT (K), K = 1, NCAB)	Card	16
READ (5,2) (W	C(K), K=1, NCAB)	Card	17
READ (5,4) (CN	M(K), K=1, NCAB	Card	18
READ (5,3) (TF	REF(K), K = 1, NCAB)	Card	19
READ (5,5) (C1	I(K), K=1, NCAB	Card :	20
READ (5,2) (C2	2(K), K=1, NCAB	Card :	21
READ (5,2) (CI	NT(K), K=1, NCAB)	Card :	22
READ (5,2) (WI	BD(K), K=1, NCAB)	Card :	23
READ (5.2) (CI	DABX (K), K = 1, NCAB)	Card .	24
READ (5,2) (CI	DABY(K), K = 1, NCAB)	Card :	25
READ (5,2) (XI	MBV(K), K = 1, NCAB)	Card :	26
READ (5,2) (Y)	MBV(K), K = 1, NCAB)	Card :	27
	Y(1), I = 1, NCUR	Card :	28
	CK (1), I = 1, NCUR)	Card .	29
READ (5.2) (PH	HD (1), 1 = 1, NCAB)	Card :	30

READ (5.3) (TENI (1) , $i = 1$,	NCAB)	Card 31
READ $(5,2)$ (XPI (1) , $1=1$, N	iCAB)	Card 32
READ $(5,2)$ (YPI (1) , $1=1$, N	ICAB)	Card 33

1000 CONTINUE

The corresponding FORMAT statements are:

- 1 FORMAT (2413)
- 2 FORMAT (8F10.4)
- 3 FORMAT (8F102)
- 4 FORMAT (8F10.6)
- 5 FORMAT (8F10.0)
- 301 FORMAT (20A4)

Subroutine BUOY

READ (5,1) CDASY, WAS, RWY, RTX, RTY, YCG, BIN (ard 34) READ (5,1) XSI, ZETI, SYDI, XPSI, ZTPI, SYPDI Card 35

The corresponding FORMAT statement is:

1 FORMAT (8F10.4)

DEFINITION OF INPUT VARIABLES FOR MAIN PROGRAM

NCASES	Number of cases, NCASES ≥ 1
TITLE	Title
NSM ²	Number of surface motion components, $1 \le NSM \le 20$
NSW ³	Number of surface wave components, $1 \le NSW \le 20$
NCAB	Number of cable segments, $2 \le NCAB \le 50$
NCUR	Number of current profile points, $2 \le NCUR \le 10$
MTRC	MTRC ≤ 0 if input data are entered in English units; MTRC ≥ 1 it input data are entered in metric units
ITER	Iteration index
$FSM(K)^2$ $AXSM(K)^2$	$x_{SM} = \sum_{k=1}^{NSM} AXSM(K)*cos(-2\pi*FSM(K)*t+FIDSM(K)*\pi 180.)$
$AYSM(K)^2$ FIDSM(K)?	$y_{SM} = \sum_{K=1}^{NSM} -AYSM(K)*sin (-2\pi*FSM(K)*t + FIDSM(K)*\pi/180.)$
ASW(K) ³	$x_{SW} = \sum_{k=1}^{NSW} ASW(K)^* \cos(-2\pi^* FRSW(K)^* t + FIDSW(K)^* \pi' 180.)$

$FRSW(K)^3$	NSW A SWAKE	/ 2 5 FROW/W FIROW/W. + /100.
F!DSW(K) ³	$y_{SW} = \sum_{k=1}^{N} -ASW(K)^* s_1$	$1 - 2\pi^{2} FRSW(K)^{*}t + FIDSW(K)^{*}\pi/180.$

RHO Fluid density in slugs/feet³ (kilograms/meters³)

SUBM⁴ Submergence of top point of cable below free surface in feet (meters)

TWX⁴ Horizontal force acting at top of cable in pounds (newtons)

TIY Vertical component of tension at top of cable in pounds (newtons)

CDASX⁴ Drag area of surface buoy perpendicular to the x-axis in ieet² (meters²)

AMC Added mass coefficient of cable; AMC = 1.0 for round cable

AFAC Cross-sectional area of cable = AFAC* π d²/4; AFAC = 1.0 for round cable TMIN Minimum algebraic tension which can be supported by cable in pounds

(newtons)

TINV! Initial time interval in seconds for dynamic calculations

DT1 Time step in seconds for which printout is desired for $0 \le t \le TINV1$

TOTT Total time in seconds for which dynamic calculations are desired

DT2 Time step in seconds for which printout is desired for TINV1 $< t \le TOTT$

DIR < 0. if initial conditions are prescribed at the bottom (towing cable

case); otherwise DIR ≥ 0

TBH Applied force in pounds (newtons) on lower weight, body NCAB-i, in x-

direction

TBYMX Maximum absolute value in pounds (newtons) of tension in cable just below

buoy; for buoy-cable system, set TBYMX equal to a large number, say, 99999

FLC(K) Length of Kth cable segment in feet (meters)

DCI(K) Diameter of Kth cable segment in inches (centimeters)

CDN(K) Normal drag coefficient of Kth cable segment

CDT(K) Tangential drag coefficient of Kth cable segment

WC(K) Weight in fluid in pounds/foot (newtons/meter) of Kth cable segment at the

reference cable tension

CM(K) Mass of Kth cable segment in slugs/foot (kilograms/meter) at the reference

cable tension

TREF(K) Reference tension in pounds (newtons) of Kth cable segment

 $C1(K)^5$, C2(K). Tension = TREF(K) + $C1(K)^* \epsilon^{C2(K)}$ + CINT(K)* $\dot{\epsilon}$; for linearly elastic

CINT(K) material, C1(K) = AE and C2(K) = 1

WBD(K) Weight in fluid of Kth body in pounds (newtons)

CDABX(K)⁶.

CDABY(K)⁶ Drag area of Kth body in feet² (meters²) for flow in (x, y) directions

 $XMBV(K)^6$. Virtual mass (mass + added mass) in slugs (kilograms) of Kth body in (x, y)

YMBV(K)6 directions

YY(1)⁷ Value of y in feet (meters)

CCK(1) Value of current in knots (meters/second) at y = YY(1)PHID(1)⁸ Initial value of ϕ of 1th cable segment in degrees

TENI(1)⁸ Initial value of tension of 1th cable segment in pounds (newtons)

XPI(1) Initial value of \dot{x} of 1th node in feet/second (meters/second)

YPI(1) Initial value of \dot{y} of 1th node in feet/second (meters/second)

DEFINITION OF INPUT VARIABLES FOR SURFACE BUOY

CDASY	Drag area for y-direction in feet ² (meters ²)
WAS	Weight in air in pounds (newtons)
RWY	Vertical distance of wind loading center of pressure from buoy center of gravity YCG in feet (meters)
RTX, RTY	(x, y) distance of cable attachment point from YCG in feet (meters)
YCG	Submergence of center of gravity below the free surface under the action of its own weight in air WAS and the vertical component of the steady-state tension (-TIY) in feet (meters)
BIN	Moment of inertia in air about YCG in slug feet ² (kilogram meters ²)
XSI, ZETI, SYDI ⁹	Initial values of (x, ζ, ψ) in (feet, feet, degrees)(meters, meters, degrees), where ζ is the vertical displacement of the center of gravity from its equilibrium value YCG
XPSI, ZTPI, SYPD1	Initial values of $(\dot{x}, \dot{\xi}, \dot{\psi})$ in (feet/second, feet/second, degrees/second) (meters/second, meters/second, degrees/second)

EXPLANATORY NOTES

- 1. ITER = 0, no iteration (prescribed initial steady-state conditions)
 - 1, free-floating cable system
 - 2, moored cable with given length in given depth
 - 3, iteration scheme to be programmed by user
- 2. For 1000. \leq FSM(1) \leq 2000., the program makes the prescribed surface motion components equal to the surface wave components by setting AXSM(K) = AYSM(K) = ASW(K), FSM(K) = FRSW(K), and FIDSM(K) = FIDSW(K) for K = 1 to K = NSM; the program automatically sets NSM = NSW.

For 2000, \leq FSM(1) \leq 3000,, the program accepts input data for a spar buoy and considers AXSM(K) to be the cross-sectional area of the buoy in feet² (meters²) at depth AYSM(K) feet (meters) below the tree surface. AYSM(1) = 0, and AYSM(NSM) = total draft under the combined action of buoy weight in air and the vertical component of the

steady-state tension. NSM should be an odd number. The input values for F\DSM(K) may take on any values such as, say, 0.

For FSM(1) > 3000., the program accepts input data for a spheroidal buoy and considers AXSM(1) to be the radius of the buoy cross section at the free surface and AYSM(1) to be the total draft. The rest of the input values of AXSM(K) and AYSM(K) as well as all of the FIDSM(K) may take on any values, e.g., 0.

- 3. For ASW(1) > 1000., the program computes the amplitude of the ASW surface wave components by using the Pierson-Moskowitz sea spectrum. In these cases, the program considers the significant wave height in feet (meters) to be (ASW(1) 1000.) and FRSW(1) and FRSW(2) to respectively be the lower and upper frequencies of the spectrum in cycles per second. The program internally generates the phases of the wave components by considering them to be uniformly separated by 360/NSW degrees. The phase of the lowest frequency component, in degrees, is taken to be the input value of FIDSW(1).
- 4. For the case of a surface buoy (FSM(1) \geq 2000.), the program calculates the drag acting on the surface buoy due to the ocean current by taking the value of the ocean current SUBM feet (meters) below the free surface. Thus, $0 \leq SUBM \leq total$ draft.

The total horizontal force at the top point of the cable TIX = TWX + $(1/2)\rho^*$ CDASX* CCF (SUBM)* ABS (CCF (SUBM)). In cases where there is no surface buoy (i.e., prescribed surface motion), TWX and/or CDASX may be set equal to zero. For cases of a surface buoy, TWX represents the wind loading on the buoy in pounds (newtons).

- 5. For free-floating and towing cables where the last (K = NCAB) cable connecting the lower weight to the ocean bottom is fictitious, read in a value for C1(NCAB) less than 0.0001 pounds (0.0004 newtons). In these cases, the program sets DCI(NCAB) = CDN(NCAB) = CDT(NCAB) = WC(NCAB) = CM(NCAB) = CINT(NCAB) = 0, FLC(NCAB) = 2 + FLC(NCAB 1), and C2(NCAB) = 1.
- 6. If CDABX(K) is negative, the program considers the body to be a circular disk with plane perpendicular to the x-axis and calculates drag and added mass forces by using the formulation given in Equation (55). In these cases, CDABX(K) is the negative of the actual drag area and XMBV(K) is the mass (not the virtual mass) of the disk. In these cases, CDABY(K) and YMBV(K) should be positive and retain the definitions given previously. Similar remarks apply if CDABY(K) is read in as a negative number except that the plane of the disk is now perpendicular to the y-axis.
- 7. When ITER = 2, the porgram takes YY(NCUR) to be the ocean depth.
- 8. For $|PHID(1)| \ge 360$, the program takes the initial values of the angle and tension of each cable segment to correspond to their respective steady-state values at the midpoint of

each segment. These steady-state values have been previously calculated by the program. This approach will minimize transient dynamic effects. In these cases, input values for the remaining PHID(K) as well as all of the TENI(K) may be arbitrary, e.g. O.

9. For SYDI \geq 360, the program sets the initial value for buoy inclination ψ equal to the steady-state value of ψ , which has previously been calculated by the program. This will tend to minimize transient dynamic motions of the surface body.

PROGRAM STORAGE AND TIME REQUIREMENTS

On the CDC 6700 currently in use at the Center, the program requires a memory of approximately 47,200 octal words to load and 33,700 octal words to execute. Compilation time is approximately 23 seconds. Execution time for a particular case depends on a large number of factors, the most important of which include the tension-strain relation of the cable, the number of cable nodes, the frequencies of the exciting surface waves and the prescribed surface motion (if any), and the amount of time over which the dynamic motions are desired. Table 2 shows the computer execution time ET and cost for all of the sample problems presented in the following chapter. A computer priority (CP) of P2 indicates overnight priority whereas P3 is the standard daytime priority at the DTNSRDC Computer Center.

TABLE 2 – COMPUTER EXECUTION TIMES AND COST FOR ALL THE SAMPLE PROBLEMS

		C1				Total
		lb		ET		Cost
Prob.	NCAB-1	(4.45N)	<u>cz</u>	sec	<u>ଫ</u>	<u> </u>
1A	4	2.4×10^{1}	1.0	61.7	P2	9.93
1B	4	2.4×10^{2}	1.0	57.8	P2	9.56
1C	4	2.4×10^3	1.0	62.4	P2	9.99
1D	4	2.4×10^4	1.0	85.0	P2	12.00
1E	4	2.4×10^5	1.0	250.0°	P2	28.00°
1F	4	2.4×10^3	0.5	1000.0°	P2	100.00*
1G	4	2.4×10^{3}	2.0	55.2	P2	8.93
2A	1	2.4×10^3	1.0	9.3	P2	5.15
2B	2	2.4×10^3	1.0	25.1	P2	6.60
2C	4	2.4×10^3	1.0	62.4	P2	9.99
2 D	8	2.4×10^3	1.0	180.0°	P2	20.00°
2E	15	2.4×10^3	1.0	600.0°	P2	60.00°
3A	4	2.0×10^{1}	1.0	90.0**	P3	15.00**
38	4	2.0×10^{1}	1.0	100.0**	P3	16.00°°
3C	4	2.0×10^{1}	1.0	100.0**	P3	16.00**
4A	4	••••	••••	82.6	P3	14.27
4 B	4	••••	••••	74.2	P2	11.10
4C***	4	••••	••••	100.0°	P3	16.00°

^{*} Extrapolated to 50 sec of dynamic motion

SAMPLE PROBLEMS

Input cards are listed for the four sample problems presented to illustrate use of the program. Representative portions of the program output are listed for one of the cases of Problem 1, but only some final results are shown for the other three problems.

PROBLEM 1 - UNIFORM CABLE WITH VARIOUS TENSION-STRAIN RELATIONS

Problem: Compute the dynamic behavior of a cable suspended from an ocean platform in the presence of a uniform current of 1 knot in the \pm x-direction for different values of the elastic constants C_1 and C_2 appearing in Equation (47)

^{**} Extrapolated to 20 sec of deployment time

^{***} Six surface wave components

^{****} See Figure 11

Case	C1 lb (4.45 N)	C2
A	2.4 x 10	1.0
В	2.4×10^2	1.0
C	2.4×10^3	1.0
D	2.4×10^4	1.0
E	2.4×10^5	1.0
F	2.4×10^3	0.5
G	2.4×10^3	2.0

The fixed surface motion, surface wave, cable, and lower body parameters are as follows: Surface motion = surface wave:

number of components	î
frequency	0.1 cps
amplitude in x- and y-directions	10 ft (3.05 m)
phase angle	0 deg

Cable:

length	1000 ft (305 m)
diameter	0.2 in. (0.508 cm)
normal drag coefficient	1.1
tangential drag coefficient	0.00
weight in fluid	0.01 lb ft (0.146 N m)
mass	0.001 slugs ft (0.0478 kg/m)
reference tension	25 lb (111.2 N)
internal damping coefficient	0

Lower weight:

weight in fluid	20 lb (89 N)
drag area in x-direction	0.3 ft ² (0.0279 m ²)
drag area in y-direction	0.3 ft ² (9.0279 m ²)
virtual mass in x-direction	1.0 slugs (14.6 kg)
virtual mass in y-direction	1.0 slugs (14.6 kg)
	1.94 slugs/ft ³ (1000.6 kg/m ³)

Fluid density:

Represent the cable by four equal segments, not including the fictitious cable segment below the lower weight. Dynamic motions are desired for a total of five cycles of the surface motions, i.e. 50 sec. For the initial interval of 10 sec. print out the transient motions every

0.25 sec. For the final 40 sec, increase the printout interval to 1.0 sec. Let the cable start from rest with the initial angle and tension of each segment equal to the steady-state values. Solution: The data cards for this problem are listed in Table 3. The cards which are the same for all the cases are listed at the top of the table. The cards for the title and the elastic constants C1 and C2, which differ for each case, are listed at the bottom of the table. The symbol b denotes a blank in this and subsequent tables which list data cards for the sample problems. Also, Column 1, 11, 21, 31, 41, 51, 61, and 71 have been indicated since most of the data start in these columns.

TABLE 3 - INPUT DATA FOR SAMPLE PROBLEM 1

	1	11	21	31	41	51	61	71
Card 1	bb1							
Card 3	bb1bb1bb	5662660			}		'	
Card 4	0.1							
Card 5	10.0							
Card 6	10.0							
Card 7	0.							
Card 8	10.0							
Card 9	0.1							ĺ
Card 10	0.							
Card 11	1.94	0.	0.	-50.	0.	1.0	1.0	0.
Card 12	10.	0.25	50.	1.0	-1.0	0.	99999.	
Card 13	250.	250.	250.	250.				
Card 14	0.2	0.2	0.2	0.2		ļ		
Card 15	1.4	1.4	1.4	1.4				
Card 16	0.02	0.02	0.02	0.02				
Card 17	0.01	0.01	0.01	0.01	}			
Card 18	0.001	0.001	0.001	0.001				
Card 19	25.	25.	25.	25.	1			
Card 22	0.	0.	0.	0.				
Card 23	0.	0.	0.	20.				
Card 24	0.	0.	0.	0.3				
Card 25	0.	0.	0.	0.3				
Card 26	0.	0.	0.	1.0				
Card 27	0.	0.	0.	1.0				
Card 28	0.	10000.						
Card 29	1.	1.	<u> </u>	<u> </u>	<u> </u>	L		<u> </u>

TABLE 3 - (continued)

	1	11	21	31	41	51	61	71
Card 30	999.	0.	0.	0.				
Card 31	0.	0.	0.	0.				
Card 32	0.	0	0.	0.				
Card 33	0.	0.	0.	0.				
Card 2	Card 2 bbb PROBLEM 1A, C1 = 24, C2 = 1							
Card 20	24.	24.	24.	24.				
Card 21	1	1.	1.	1.				
Card 2	bbb PRO	BLEM 1B, C	C1 = 240, C	2 = 1				
Card 20	240.	240.	240.	240.				
Card 21	1.	1.	1.	1.				
Card 2	bbb PRO	BLEM 1C, C	C1 = 2400,	C2 = 1				
Card 20	2400.	2400.	240 0.	2400.				
Card 21	1.	1.	1.	1.				
Card 2	bbb PRO	BLEM 1D, (C1 = 24000	. C2 = 1				
Card 20	24000.	24000.	24000.	24000.				
Card 21	1.	1.	1.	1.			_	
Card 2	bbb PRO	BLEM 1E, C	c1 = 240000), C2 = 1				
Card 20	240000.	240000.	240000.	240000.				
Card 21	1.	1,	1	1.				
Card 2	bbb PRO	BLEM 1F, C	C1 = 2400,	C2 = 0.5				
Card 20	2400.	2400.	2400.	2400.				
Card 21	0.5	0.5	0.5	0.5				
Card 2	bbb PRO	BLEM 1G.	C1 = 2400.	C2 = 2		- <u> </u>		
Card 20	2400.	2400.	2400.	2400.				
Card 21	2.0	2.0	2.0	2.0				

Table 4 shows the first six and the last two pages of the computer output for Problem 1D. The pages which have been left out simply contain output for the dynamic motions for intermediate time intervals. Table 4 shows that the first page of the output lists a table of conversion from English to metric units. The second page lists the input data. For the present case of a suspended cable, where the initial conditions are known at the lower body, two integrations are performed for the ste. A state configuration (see section on Initial Value Cascs). The results of these two integrations are given in the next two pages. The remainder of the output is a listing of the dynamic displacements, velocities, and accelerations of the surface waves, upper cable point, and each node at the prescribed time intervals. The output also lists the angle, angular velocity, tension, strain, and strain rate of each cable segment.

TABLE 4 - PROGRAM OUTPUT FOR PROBLEM 1D

CONVERSION FROM FNGLISH UNITS TO METRIC UNITS

ENGLISH METRIC

1 7NCY= 2.540 CF

1 50 FT= 0.09290 SQ M

1 CU FT= 0.02041 CU M

1 POUND= 4.452 NEWIONS

1 SCOND= 1.60 V NEMIONS

1 SECOND= 1.500 V

1 KNOT= 0.5144/SEC

PROBLEM 10.01=24888.02=1

								5444
								600 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
								2004 PROPERTY CONTROL OF CONTROL
								Z00000 Z00000 T00000 T00000
		_						21°C2 24000 24000 24000 24000 24000 24000 24000
RISTICS		ML(FT) MK(1/FT) 512.40 .0123			99			2000 2000 2000 2000 2000 2000 2000 200
CHARACTE		812.48			PHASE (04			151/F) 7 1100 1100 1100 1100 1100
LISTING OF ENVIRONMENTAL AND CABLE-BUDY CHARACTERISTICS					V-A(FT) PHASE(OEG)			M(LO/F) M(SL/F) 7 REF(LD) -0100 -00100
AL AND CA		FREGICES) ANDLIFT: PHASE(0)	CURRCATO 1.10 1.10	:	SUAFACE MOTION-FREGICPS) K-Aiff)			
V IRONNENT		1CPS) AND	DEPTH(FT) 0.00 10001	1.9488 SL/GUFT	.1965	RISTICS	=	CABLE PROPERTIES 1 LEMIFT) DIAMITMY CON 2 259-88 .2888 1.4888 2 259-88 .2888 1.4888 4 259-88 .2888 1.4088 5 588-88 .2888 1.4088 5 588-88 6.8888 1.4088
NG OF EM	111 045	FRES			1 10m-fae	CHARACTE A FAC T		A 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
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	3	SURFAC	CURREN	61019	ಪ	CENERA AN CC	3	3 2 2 3

200000

6.2832

YVA

. 0000

X V D =

INITIAL VALUES AT TOP OF CABLE

SUBMIFT) MIND LDILB) CDAXIFTSD) MAK TEMILB) 6.8088 8.8888 8.8088 99999.8088

SURFACE BUDY CHARACTERISTICS

::::

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K-LOAD OM 801

TABLE 4 - (continued)

QUN NUMBER 1

STEADY-STATE CONFIGURATION

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20.00	, ,,,,,						-163.82							+2.504-	-573.50	19 109	70.7001	16.88.63		-24.09 LB	-12.55 18	-24.0654 LB		
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TABLE 4 - (continued)

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-	25	0	25.0	120.1	9.1	4.9	•
	5	0	50.0	228.3	21.6	5.7	•
-	50	0	50.0	226.3	21.6	5.7	٠
~	2	0	75.0	333.9	99.6	5.3	٠
~	30	0	000	435.2	61.7	4.2	•
~	8	0	0.00	435.2	61.7	4.2	•
*	629.	00	625.01	529	343.41	23.33	١
m	53	0	50.0	612.9	36.6	2.3	•
m	50	0	50.0	612.9	36.6	2.3	٠
	75	ō	74.9	675.6		1.2	٠
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	900	ō	99.9	703.6	65.4	•	
*	53	~	279.4	454.6	46.8	•	
•	530	•	56.9	145.6	32.1		
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A-CO	F-COMPONENT	P	TENS ION=	00 18			
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TABLE 4 - (continued)

COMPUTED CABLE SYSTEM MOTIONS AND TENSIONS

2	1 (566)	5	K(FT)	1 (7)	KP (F1/5)	YP (FT/S)	XPP (F /55)	£/3	7EH (L.B.)	F1(0EC)	FIP (0/S)	FIIDEG) FIPIO/S) STRA'H STP	(71)4
>-~~~ 1	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		22.00 6.10 6.10 6.10 7.10 7.10 7.10 7.10 7.10 7.10 7.10 7	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	2000 2000 2000 2000 2000 2000 2000 200	-11.1273 -11.1273 -11.0352 6.7333 5.000	-6-6133 -10-2609 -12-2671	7000	**************************************	1.40.40.40.40.40.40.40.40.40.40.40.40.40.	1.70000115 . 	00000 00000 00000 00000
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3 4 90 - 4 M M 4			6.09 230.00 437.05 616.03	25.73 26.73 635.73	-3.6932 -3.6932 -3.6932 -3.6932 -3.6932 -3.6932 -3.6932	5.0032 5.0032 -1.1133 -1.1133 -1.1133 -1.1133	-3.1939	\$ 262 \$ 262 \$ 2662 \$ 1622 \$ 1622	1776	15555 1555 1	**************************************	100000 10000 100	13140 12296 13075 13075
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# # # # # # # # # # # # # # # # # # #		015625 015625 015625 015625 015625	6.00 6.00 6.00 6.00 6.00 6.00 6.00 6.00	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	00000000000000000000000000000000000000	3.6982 12.2264 11.7585 12.7732 13.5007	64. 64. 666.	60 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0	\$6.50 \$6.50 \$6.50 \$6.50 \$6.50	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	N P 4 4 5 5 4 N 4 P 7 1 1 1	. 100931 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0 .	00000000000000000000000000000000000000
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TABLE

TABLE 4 - (continued)

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Figure 5 shows the tension in the upper cable segment for $0 \le t \le 20$ for Cases 1A to 1E, where C1 increases from 2.4 x 10 lb (1.07 x 10^2 N) to 2.4 x 10^5 lb (1.07 x 10^6 N). As would be expected, the figure shows that tension fluctuations increase with increasing values of C1. At the highest value of C1 = 2.4 x 10^5 lb (1.07 x 10^6 N). Figure 5 shows that the tension is zero at t = 0.25 sec (i.e., the cable in slack) and then jumps to a value of 89.47 lb (398.1 N) at t = 0.5 sec. This sudden jump in tension after a slack condition is often referred to as "snap loading." At the other extreme, the tension shows a peak-to-trough fluctuation of less than 2.5 lb (11.1 N) at the lowest value of C1 = 2.4 x 10 lb (1.07 x 10^2 N).

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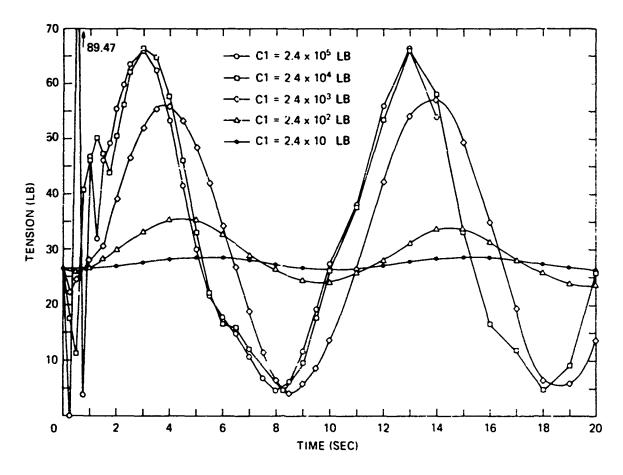


Figure 5 - Tension in Upper Cable Segment for Various Values of Cl

Table 2 indicates that the computer execution time remains at approximately 60 sec for Cases 1A, 1B, and 1C for which $C1 \le 2.4 \times 10^3$ lb (1.07 x 10⁴ N). At higher values of C1, Cases 1D and 1E, execution time increases with increasing C1. Execution time also increases with decreasing values of C2, Case 1F. The reasons for these increases may be most conveniently explained by considering Equation (9) which shows that for a given value of

 $(T - T_0)$, ϵ decreases with increasing values of C1 and decreasing values of C2. Small values of ϵ , in turn, mean that the x- and y-displacements must be calculated with greater precision, leading to smaller integration time steps.

PROBLEM 2 - UNIFORM CABLE REPRESENTED BY VARIOUS NUMBERS OF SEGMENTS

Problem: Solve Problem 1D with the single exception of representing the cable by the following number of nodes, NCAB-1: 1, 2, 4, 8, and 15. Consider the nodes to be equally spaced.

Solution: The data cards for this problem are listed in Table 5. Cards 1 and 4 to 12, which are identical to those for Problem 1D, are omitted. Also omitted are Cards 14 to 33 which are similar to those for Problem 1D with the exception that the number of entries depends on NCAB.

TABLE 5 - INPUT DATA FOR SAMPLE PROBLEM 2

	1	11	21	31	41	51	61	71
Card 2	bbb PRO	BLEM 2A,	NCAB-1 = 1					
Card 3	bb1bb1b	b2bb2						
Card 13	1000.							
Card 2	bbb PRO	BLEM 2B,	NCAB-1 = 2					
Card 3	bb1bb1b	b3bb2						
Card 13	500 .	500.						
Card 2	bbb PRO	BLEM 2C,	NCAB-1 = 4					
Card 3	bb1bb1b	b5bb2						
Card 13	250.	250.	250 .	250 .				
Card 2	bbb PRO	BLEM 2D.	NCAB-1 = 8	1				
Card 3	bb1bb1b	b9bb2						:
Card 13	125.	125.	125.	125.	125.	125.	125.	125.
Card 13	bbb							
Card 2	bbb PRO	BLEM 2E,	NCAB-1 = 1	5				
Card 3	bb1bb1b	16bb2						
Card 13	66 667	66.667	66 667	66.667	66.667	66.667	66.667	66.667
Card 13	66.667	66.667	66.667	66.667	66.667	66.667	66.667	

Figure 6 shows how the steady-state location of the lower weight varies with NCAB-1. The results are poor for NCAB-1 \leq 2, good for NCAB-1 = 4, and converged for NCAB-1 \geq 8. Figure 7 shows the velocities \hat{x} and \hat{y} of the lower unit at t = 4 and 10 for various values of NCAB-1. The results suggest that a minimum of four nodes is required to obtain maximum.

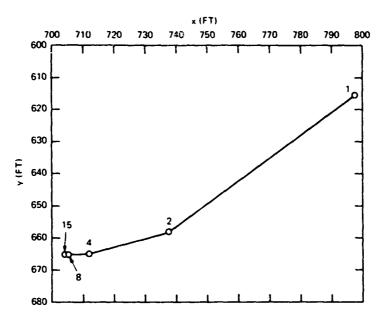


Figure 6 - Steady-State Location of Lower Weight for Various Numbers of Nodes

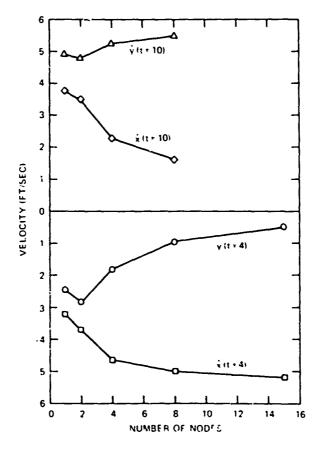


Figure 7 - Velocity of Lower Weight for Various Numbers of Nodes

velocities with accuracies to within 10-20 percent. The magnitudes of the computed velocities generally increase with increasing number of nodes.

Table 2 shows the manner in which the computer execution time ET increases with the number of nodes. The execution times are approximately proportional to NCAB squared

$$ET \alpha (NCAB)^2$$
 (56)

PROBLEM 3 – DEPLOYMENT OF SUSPENDED CABLE IN CIRCULATING WATER CHANNEL

Problem: Compute the deployment of an initially vertical cable, suspended in the DTNSRDC Circulating Water Channel, to its final steady-state configuration in the presence of channel flow speeds of 1, 2, and 4 knots (0.515, 1.03, 2.06 m/s). The fixed cable and lower body parameters are as follows:

Cable:

length	12 ft (3.66 m)
diameter	0.12 in. (0.305 cm)
normal drag coefficient	1.4
tangential drag coefficient	0.02
weight in fluid	0
mass	0.0002 slugs/ft (0.00957 kg/m)
reference tension	2 lb (8.9 N)
C_1	20 lb (89. N)
C ₂	1.
internal damping coefficient	0.
eight:	

Lower weight

weight in fluid	2 lb (8.9 N)
drag area in x-direction	0.05 ft ² (0.00465 m ²)
drag area in y-direction	$0.05 \text{ ft}^2 (0.00465 \text{ m}^2)$
virtual mass in x-direction	0.1 slugs (1.46 kg)
virtual mass in y-direction	0.1 slugs (1.46 kg)
Fluid density:	1.94 slugs/ft ³ (1000.6 kg/m ³)

Represent the cable by four equal segments, not including the fictitious cable segment below the lower weight. Take the total time interval to be 20 sec. For the initial interval of 1 sec, printout of the transient cable configuration is desired every 0.1 sec. For the final 19 sec, increase the printout interval to 0.2 sec. Let the system start from rest with the initial tension in each cable segment equal to 2 lb (8.9 N).

Solution: The data cards for this problem are listed in Table 6. The cards which are the same for all the cases are listed at the top of the table. The cards for the title and the current magnitude which differ for each case are listed at the bottom of the table.

TABLE 6 - INPUT DATA FOR SAMPLE PROBLEM 3

	1	11	21	31	41	51	61	71
Card 1	bb1							
Card 3	bb1bb1bb	5bb2						
Card 4	0.1							
Card 5	0.							
Card 6	0.							
Card 7	0.							
Card 8	0.							
Card 9	0.1					}		
Card 10	0.							
Card 11	1.94	0.	0.	- 15.	0.	1.0	1.0	0.
Card 12	1.	0.1	20.	0.2	-1.	0.	99999.	
Card 13	3.	3.	3.	3.				
Card 14	0.12	0.12	0.12	0.12				
Card 15	1.4	1.4	1.4	1.4		ĺ		
Card 16	0.02	0.02	0.02	0.02				
Card 17	0.	0.	Э.	0.				
Card 18	0.0002	0.0002	0.0002	0.0002				
Card 19	2.	2.	2.	2.				
Card 20	20.	20.	20.	20.				
Card 21	1.0	1.0	1.0	1.0				
Card 22	0.	0.	0.	0.				
Card 23	0.	0.	0.	2.				
Card 24	0.	0.	0.	0.05			į	
Card 25	0.	0.	0.	0.05				
Card 26	0.	0.	0.	0.1				
Card 27	0.	0	0.	0.1				
Card 28	0.	100.						
Card 30	0.	0.	0.	0.				
Card 31	2.	2.	2.	2.				
Card 32	0.	0.	0.	0.				
Card 33	0.	0.	0.	0.				

TABLE 6 - (continued)

	1	11	21	31	41	51	61	71
Card 2	bbb PR	OBLEM 3A	, V = 1 Kñ	OT		-		
Card 29	1.0	1.0						
Card 2	bbb PR	OBLEM 3B	, V = 2 KN	OTS				
Card 29	2.0	2.0						
Card 2	bbb PR	OBLEM 30	, V = 4 KN	отѕ				
Card 29	4.0	4.0						

Figures 8, 9, and 10 show the configuration of the cable nodes at various times during deployment for currents of 1, 2, and 4 knots (0.515, 1.03, 2.06 m/s), respectively. These figures show that approximately 8 sec are required to reach the steady-state configuration.

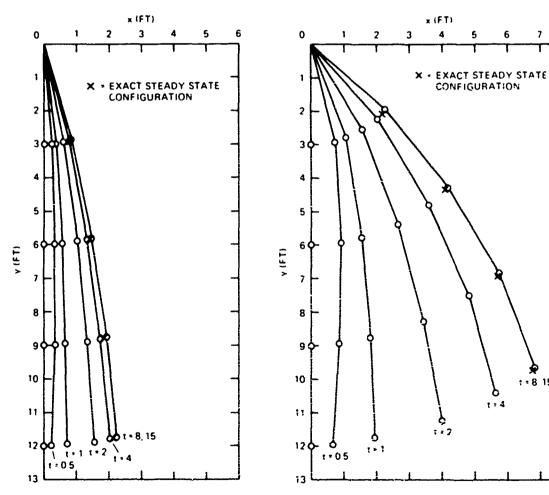


Figure 8 - Configuration of Nodes at Various Times during Deployment. C = 1 Knot

Figure 9 - Configuration of Nodes at Various Times during Deployment. C = 2 Knots

: *8 15

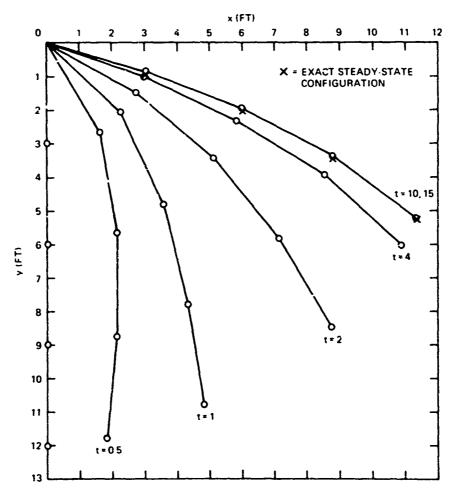


Figure 10 - Configuration of Nodes at Various Times during Deployment, C = 4 Knots

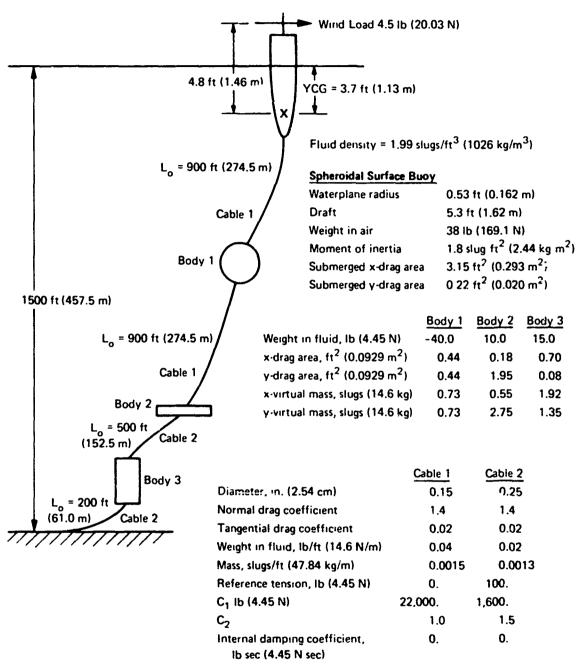
In all three cases, the final deployed configuration modeled by the four nodes was in close agreement with the exact configuration calculated by using the differential equations (1) to (5). It is of interest to note that because of its large inertia, the lower weight characteristically lags during the initial instants of deployment.

Table 2 shows that the computer execution time for a total of 20 sec of deployment time, which is well in excess of the 8 sec required to reach the steady-state configuration, is approximately 100 sec.

PROBLEM 4 - COMPLETE MOORED BUOY-CABLE-BODY SYSTEM

Problem: Compute the dynamic motions for the moored buoy-cable-body system, with parameters as shown in Table 7.

TABLE 7 - PARAMETERS FOR MOORED BUOY-CABLE-BODY SYSTEM



Current Profile:	Depth	Current
	(ft)	(knots)
	(0.305 m)	(0.515 m/s
	0.	2.50
	500 .	1.39
	1000.	0.50
	1500.	0.50

The upper body is a buoyancy bag, the middle body is a damping disk, and the lower body is an acoustic unit. Consider the following three cases:

Case A. Use the formulation contained in the program for a spheroidal buoy. Let the surface wave be composed of a single component with frequency = 0.1 cps, amplitude = 7.5 ft (2.29 m), and phase = 0 degrees.

Case B. Same as Case A except that the buoy is read in as a spar buoy, modeled by 11 cross-sectional areas as a function of depth from 0 to 5.3 ft (1.62 m).

Case C. Same as Case A except that the surface wave amplitudes are to be calculated by using the Pierson-Moskowitz sea spectrum for six components. Take the significant wave height as 15 ft (4.58 m), the range of frequencies from 0.04 to 0.28 cps, and the phase of the lowest frequency component equal to -60 deg.

For all three cases, model the cable by 4 nodes, with three of them corresponding to the three intermediate bodies and the extra node 200 ft of cable below the surface buoy. Compute the dynamic motions for 50 sec. For the initial 10 sec, print out the dynamic motions every 0.1 sec. For the final 40 sec, increase the printout interval to 0.5 sec. Let the system start from rest with the initial angle and tension of each segment equal to the steady-state values. Let the initial x-displacement of the buoy center of gravity be 7.5 ft (2.29 m).

Solution: The data cards for this problem are listed in Table 8. Again, cards which are the same for all the cases are listed at the top of the table. The cards for the title surface buoy, and surface waves, which differ for each case, are listed at the bottom of the table.

Figure 11 shows the pitch angle ψ of the surface buoy for all three cases for $20 \le t \le 40$. Perhaps the principal feature is the comparison of the result for the same surface buoy treated as a spheroidal buoy and as a spar buoy. The figure shows that when the spheroidal buoy formulation was used, pitch results were about 1 to 2 deg higher than those obtained from the spar buoy formulation. This discrepancy is due to the difference in the added mertia terms for the two formulations (compare Equations (27), (28), (32) and Table 1).

The figure also shows that results for the single frequency cases 4A and 4B exhibit a periodic behavior with a period of 10 sec; the results for the multifrequency case 4C show a more random behavior.

Table 2 shows that the execution times for the single-frequency cases 4A and 4B were approximately 80 sec. This time was increased to 100 sec for Case 4C, where the program must calculate six components to obtain the surface wave.

TABLE 8 - INPUT DATA FOR SAMPLE PROBLEM 4

	1	11	21	31	41	51	61	71
Card 1	bb1							
Card 11	1.99	0.	4.5	-100.	3.15	1.0	1.0	0
Card 12	10.	0.1	50.	0.5	1.0	0.	99999.	
Card 13	200.	700.	900.	500.	200.			
Card 14	0.15	0.15	0.15	0.25	0.25			
Card 15	1.4	1.4	1.4	1,4	1.4			
Card 16	0.02	0.02	0.02	0.02	0.02		ļ	
Card 17	0.04	0.04	0.04	0.02	0.02			
Card 18	0.0015	0.0015	0.0015	0.0013	0.0013			
Card 19	0.	0.	0.	100.	100.			
Card 20	22000.	22000.	22000.	1600.	1600.			
Card 21	1.0	1.0	1.0	1.5	1.5			
Card 22	0.	0.	0.	0.	0.		}	
Card 23	0.	-40.	10.	15.	0.			
Card 24	0.	0.44	0.18	0.70	0.			
Card 25	0.	0.44	1.95	0.08	0.			
Card 26	0.	0.73	0.55	1.92	0.			!
Card 27	0.	0.73	2.75	1.35	0.			
Card 28	0.	500.	1000.	1500.				
Card 29	2.50	1.30	0.50	0.50				
Card 30	9993.	0.	0.	0.	0.			
Card 31	0.	0.	0.	0.	0.			
Card 32	0.	0.	0.	0.	0.			
Card 33	0.	0.	0.	0.	0.			
Card 34	0.22	38.	-4.8	0.	1.6	3.7	1.8	
Card 35	7.5	0.	999.	0.	0.	0.		
Card 2	bbb PRO	BLEM 4A, S	SPHEROIDA	AL BUOY,	SINGLE W	AVE FREQ	UENCY	
Card 3	bb1bb1bb	5bb4bb2						
Card 4	3500.							
Card 5	0.53							ļ
Card 6	5.3							
Card 7	0.							
Card 8	7.5							
Card 9	0.1			•				
Card 10	0.							

TABLE 8 - (continued)

	1	11	21	31	41	51		71
Card 2	bbb PRO	BLEM 4B,	SPAR BUO	Y, SINGLE	WAVE FR	EQUENCY		
Card 3	b11bb1b	o5bb4bb2						
Card 4	2700.							
Card 4								1
Card 5	0.883	0.852	0.812	0.757	ე.687	0.600	0.498	0.380
Card 5	0.247	0.097	0.					
Card 6	0.	1.0	1.5	2.0	2.5	3.0	3.5	4.0
Card 6	4.5	5.0	5.3					Ì
Card 7	0.							
Card 7	0.							
Card 8	7.5							
Card 9	0.1							
Card 10	0.							
Card 2	bbb PRO	BLEM 4C,	SPHEROID	AL BUOY,	SIX WAVE	FREQUEN	Cies	
Card 3	bb1bb6b	b5bb4bb2						
Card 4	3500.							
Card 5	0.53							
Card 6	5.3							}
Card 7	0.							
Card 8	1015.							
Card 9	0.04	0.28						į
Card 10	-60.							

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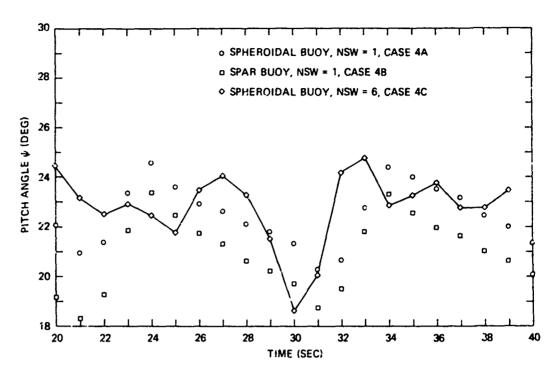


Figure 11 - Pitch of Surface Buoy

ACKNOWLEDGMENT

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APPENDIX LISTING OF COMPUTER PROGRAM

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6	大におしている。人にして「大き」というにある。大には、大には、大には、大には、大きに、大きに、大には、大きに、大きに、大きに、大には、大きに、大きに、大きに、大きに、大きに、大きに、大きに、大きに、大きに、大きに	CABUDY	7 1
	-	CABUOY	. .

P 46E

PROGRAM CABUDY	07 74/7*	7* OPT	1 = 0 K OU	=0 ROUND=+/ TRAC	TRACE	T.	FIN 4.6.420	• 5 0	04/19/77	11.49.22	
				į							
	MEAU (5.4)		CH (K) OKHIONCAU)						CABURY		
	ME AU (5.5)	1 1 1 1 1 1							CABUDY		
	KEAU (5.5)	(1112)	. K . I . NC	99					CABOOV		
	READ (5.2)	(22 (8)	2 - 1 - MC	48)					CABOOK		
	ME AD (5.2)	LENICE							100040		
	KEAJ (3.6)	1000									
	454012451	40400	4								
	MEAD (2 + C)		1 - 4 - 4 - 4								
	ME AD 15.21	**************************************									
	PEACES IN										
	READ (5.2)	PALOC	1 = 1 = (MCA B					CABUDY		
	READ(5.3)	ITENIC.	1.1.1	MCAB					CABUOY		
	AE AD (5.2)	(X) Ydx)	X. Telle	CAB					CABUOY		
	KEADIS-21 (YPIII) .I. 1.NCAB)	(101	.1 . I . I	CVB					CABUDY		
	OlkPausk								CORR		
	JO 1000 MC+1. MCASES	C: 1. NCA	SES.						CABCOY		
	OLK CLK		•	;					2 KOO		
	READ(5.3)		X . T = T . C	ŝ					CABUOY		
3	IP (MIRES LESS) 60 10 221		122 01	101 101	STATE				X 4 6 5 5	12	
	222 545	1									
	AKSM(1) = 3.	241 • AXS	(7) 85						2000		
	IF (FSM(1)	E. 2 001	AMO	FSH(1)	.LT.3008.) A	X SM (I) #3	1.281.4	LKSH(I)	CORR		
222		. 201 . AYS	SH (E)						CORR		
		.LE.1001	09	10 224					CORR		
	ASM(1)=10	90.43.24	11.14SH	(1)-10	(*90				CORR		
;	20 10 226				70 10 226				COR.		
422	CONTINUE								X (0)		
***	81 622 OC	1.15	:						2 CO		
466	7 6 6 6 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1		3,								
• 77	ATOMOTO SA		7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	/h. h.g.	47 1 V=7 1 V / h. h.	6.3					
	COAS X 3. 2	1 0 3 . 2 4 3	I COASK	A I M I M I	TMIN/4-45/81	BYET BH/4	564-0		200		
	TEYEKA TEY	4/4.45							200		
	JO 227 Km	. NCAB	ì						CORR		
	FLC(R)=3.	2010510	(K) 60CI	(x):	3937*UCI (K) Si	MC (K)	.94846	MC (K)	CORR		
	.0×(X)X0	20 47 + CM	(K) & TRE	F (K) =T	REF (K) /4.452	\$C1 (K) =C	11(X) /	452	CORR.		
	CIMT (K) #C	[N] (K)/.	1. 52 SH	10 (x) =	M30(K)/4.452				CO.		
	COABKIK	3. 28. • 3.	. 281 ° CD	ABKCK	SCOABY (K) = 3.	261.3.28	11.004	97 (X)	200		
	ATEN (A) = 0.	707000	ANDAGE.	ABLAS!		THOVER			K 4		
100		70.000									
3		- MCUR	Ì						200		
	YY (1) = 3.2	(I) AA. 18	_						CORR		
22.0	CUK(1) # 3. 241 * CCK(1) / 1. 6876	241°CCK	111/11.6	1.1					CORR		
122									CORR		
	IF (C1 (MCAE) . GT. 0. 0831) GO TO 115		.0001	0	115						
	FLCCNCABO	15.0FLC	CHCAB-1	2000	NCABLED. SCON	CNCAB) = 6					
	COLUMN			E CHO	くじしょどうかせいなっしゃないんないからしょうしゅうじょくのいちゅうせん スパートングラー・ロックグドイスのものう そのうのはいしょう カラン・コード・ファンコードウェント・ファンコード			4CA6) *6.00			
¥ -		2 7 7 4 1 7		:							
611											
	101711111111111111111111111111111111111								A01040		
	PIEST 141592656232	97.65.6.33	.,						CABUDY		
	XSI.0.		•						CABUDY		
	DKUL = 2.2/1.12	1.12							CABUDY		

	PROURAM CAUUDY 74/74 OPFEG ROUNDER/ TRAJE FIN 4.6+428	04/19/77	11. 69.22
115	4.17.11.130	*	•
:			9 5:
		CABUOY	2
	151 CONTINUE	CABUOY	: 2
	IF (AVELL) . LE. 10000.) CO TO 80	CABUDY	~
177	DO DEL MAN	CABUDY	2
	なのではなりました。	CABUOY	21
		CABCOV	۲:
	202.1.\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	CORR	52
		CABUDY	≈:
3	SOUTH THE CO.	>00843	• •
		CABUDY	
	(CABUDY	
	エアジュ(こ)との1/21/21/21	CABUOY	~
1 30		CABUOY	
	IF ((FSM1.LC.1000.).0%.(FSM1.6E.2000.)) 60 10 91	CABUDY	3
	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1000 1000 1000 1000 1000 1000 1000 100	9.
	TO THE TANK TO THE TANK THE TA	10001	5
1.35	・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・ ・	CABLOY	° •
	DON INCE	CABUDY	
	*	CABUDY	•
	C PARK CATA	CABUDY	2
		COR.	25
0 4 -	SOZ FOFMAT (1111-10x-20At)	CO	.
		X (0.00)	.
	CONTRACTOR OF TAXABLE CAR INTERNATION OF TAXABLE CONTRACTOR OF TAX	300	2 6
	AVAILE (N.T.)	CABLO	7
1.45	/ FORMATICS#.16HOGEAN COMDITIONS)	CABUOY	95
	24176 (6.0)	CABUOY	*
	* FORMAI(/11.15ALURFACE MAVE5X.44FREG(CPS).1X.6HAMPL(FT).2X.	CABUOY	44
	LAMPHASE(J) + 5x + 6MML (FT) + 2x + 8MMK (1/FT) >	CABUOY	•
:		CABUOY	\$
150		CABUDA	100
	(7-074-2-074-5-x-2-074-5-x-2-7-2-7-7-7-7-7-7-7-7-7-7-7-7-7-7-7-7	CABUOY	101
	TATE ORDINE NO TELEVISION NOT STREET TO STREET	ACTES.	
		CABUOT	
152		CABUDY	105
	11 FORMATICOX, 2F 10, 2)	CABUDY	700
		CABUDY	201
	こく そしたコラーインボタットチェア しし アンドード・ケーヴ・ドゥル スンピース・アンピース・ファーン また コープラー・カー・カー・カー・カー・カー・カー・カー・カー・カー・カー・カー・カー・カー	CABUOY	• o
164	THE CLUSTER AND CONTRACTOR FOR SOME STATE OF THE STATE OF	CARUDY	-
3	I THY-A (FT) . LX. LOHPHASE (O	CABUOY	, o , o , o
	UD 33 1-1-North	CABUOY	112
	SA MARIOTOPIA TURINO AKORANO AVORANO PRODUKI	CABCOY	113
•	CP-DKLF-WDXD-EEXOL-FT	ACCEPTO	
è	CANTAL SANCE OF SALES	CABUDY	
		CABUDY	21
	16 FORMAIL/2X.GHAM COEFF.2X.BJAREN FAL.2X.947 MIN(LB))	CABUOY	•:-
		CABUOY	•=
2 70	17 70 AND 17 (20 80 80 80 80 80 80 80 80 80 80 80 80 80	CABUDY	1 20
		CABUOA	Ç.

	PROGRAM CABUOY	CABUD	27.		ě	OPT=0 ROUND=+/ TRACE	/ TRA	ICE	Z	FIN 4.6+420		04/19/77	11.69	27 .
		•		•								1		
		•	# TF (6.19)		,		15.75	FORTER LANDS OF SETERAL FRONTER SECONDARION FRONTENDS BETTER THE SECONDARION FRONTENDS BETTER THE SECONDARION FRONTENDS	7	11 1E 57			7. 1	
			DOMATAR	N. M. M.	. 7	E M / E A		THE PARTY OF THE P	0.074	1 2 × 3 × 10	,			
. 74		•							24.57	10074091	****			
:			200000000000000000000000000000000000000	7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2			3 2 3							
			2					nterdere to the realization of the property of the second section of the second section of the second second second second section of the second seco		. C IA.				
		, 3										100000		
		, 1	100 77 71 01	3			2	1 101 000						
4		•					3		7777		•	200		
		4	DMC TANK		5	7 40 40		7.000.77.40.		AGM 1 + 7 T				
		202	OWHAT CLKAL	3.F 10.	2.461	A. h. F.	. 6. 6 :	8.2.610.0.E	4.6.				101	
		1	8-6-2K-3F	3.25								70000		
			1K17E (4-55)		•								•	
1.05		55 F	OKMAT (/1X.	17 HX-L	040	TOB MC	MI	FORMAT (/1x, 17 Hx-LOAD ON BOT MTR.F12, 4, 1x, 24LB)	(8)				3 3	
		. *	#RITE (6.59)						;			2000	5	
		5.4 6	ORMAT (// LX	,28HSU	RFACI	E 8004	CHAR	54 FORMATI//LK, 28HSURFACE BUOY CHARACTERISTICS)	_			CORR	3	
		=	IR11E (0.52)					MRITE (0.52)				CORR	65	
		25	OKHAT CZEK.	MEDSHE	(FI)	. 2x . 11	HAE NO	1 LO(LB). 2X+	18HCDAX	(FTS0)	2×.	CORR	99	
0 × 1		=	SHREK TEN	3								CORR	3	
		* '	ARITE (6.53) SUBM.TAX.COASX.TBYRK	SE SE	7 8 %	COASX	, T O Y E	_				CO.	3	
	•	^	UKRAI (47 12		•							200	5	
			VET CONVINCENT BRO CONVERT UNITS	DAMO	ONAE	140 TA	4					CABCOY	# 1	
196		. a	DACE (AB. /BT	7797								X 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	? :	
:		. 7	SELTABLE BROKEN	10000									9 P	
		, 12	COLUMN TATABLE NA											
		z z	15 M (I) = 2, 09	TOFCHE	2									
			F13H(1)-F13SH(1)-P1/148.	SM(I)	77.11	į						CABUOY		
200		3	CONT INUE			,						CABUOY	•	
			JO 42 INT.NUR	301								CABUOY	**	
		ر د د	.CF(1) =CCK(11.1.6	676							CABUOT	-	
		-	MBOT = MBO(Ndo)	3								CABUDY		
		*	4CX=0									CABCOY	145	
		- ''	10317 VV 14 0 1 1 1											
		, =	6 4 1 1 6 8 4 6	07	10 01	_						200		
	٥	ני	FORM INCL	4	Cut	LIONS	FOR	CALCULATIONS FOR ITERATION CA	SASES					
	,		UIR-1.0						;			CORR	2	-
211		25.0	dcvi.e.									CABUOY		
		_	DO SE INT. NCA	7			;					CABUDY		
		= - -	#CV 1 # # CV 1 + F LC (1) * #C (1) + ##D (1)				2					CABUDY	1 55	
		. n	30 198 I=1. NCAB	MCA 8										
212		1 36 1	TOTL = TOTL +1 .1 * FLC (1)	.1.FLC	3							CORR	7.	
		* (4 TX = - 1000 - 14 TX = 1000.	488=10	•							CORR	2	
		-	では、 はず 1mm からです 1mm 1mm	707	7 7 7 7 7 7								1 59	
		. ~	I COL (I) TI ANN ANNECOLIS				_					CABUDY	3	
229		, 	IF (YY (1) . GT . TOT L)	.101	3	TO 102						2000	:	
		_	COMT SHUE									CABUOY	163	
		105	IF (VMX-LT-6.)	02 09 (-		103						CORR	2	
		- *	MEN - 50 - 0 = NUM									2 C C C	2 6	
225			00 10 93									200	9	
		7 0 7	VMX=0.95*VMA	4								CORR	82	
			MAR 1.05. VA	z								CORR	83	
		, 57	LALL ITERA									CABUDY	164	

	PROCRAM CANCON	1/12	001 = 0 A0	OPT = ROUND= -/ TRACE	FIN 4.61420	02749	04/19/77	11.49.22	
	y	HATS-WOARTS ATA MIDIAL		22.04.042			2	;	
2.30		5) k / 1 = -1							
:							-	2	
		בשנו נפעומסשינם	9					167	
	3	22.47.47.					CABUOY	191	
		3 * O I & W I O * K	DASK.CCF	*ABS (CCF 1)			CABUDY	169	
	7.	7. X a 10 X b 1 M A					CABILDY		
2.35	***	COLP.LT.A.	71 X = 0 . 4.	CONTRACTOR SOLUTION	TOUR TOUR DESTREE	MB41140		: :	
		4.71.46.7					2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		
							CABUDA	2/1	
				2			CABOO	27	
	2	FRISHBIANCE IX 6-1 LT	X 1 LT				CABUOT	2.	
•		C. Ref &	,				CABUDY	175	
9	•	DAT OF THE STATE	۰				CABUOY	176	
		15=-11767x5=	×11-				CABUDY	111	
	1	IF (FSM1-LE.2000.) GO TO	100.1 60 1	97 0			CABUDY	176	
	1	(DIR.LT.0.)	60 10 9				CABUDY	179	
	70	CALL BUOT					CABUOY		
542		75 01 05					CABUOY	1 0 1	
	0.3 47	COMI INUE					CABUDY	1 62	
	×	XAHD. SYAL SUBMEXPAND. SYPAND	SXPA = 0. SY	PA:0.			CABUDY	103	
	90	X4X.1=1 44 0					CABUOY	181	
	77	INKANAKSH(I)	*COS(F1SH	AAAAAAAAAA IN OCOUCTION IIN			CABUOY	105	
250	7.4	CI) WS A V V A F I	ESILIEIS.	(1))			CABUOY	1 16	
	×	T) WSM. FAX . V.	JOH SX P. (*SINCFISHCIDD			CORR	57	
	44	A. YPA+MSH.	J. ATSMILL	*CDS(FISH(I))			200	7	
	200	CONTINUE					A C C C C C C C C C C C C C C C C C C C	3	
		COMP TAKE							
996		4 4 4 4 4 4 4 4 4 4 4 6 6 4 4 6 6 6 6 6	4-204					2	
:		THE CALL OF THE PROPERTY OF THE PARTY OF THE		• •			200	>	
		11.EK-66-17	2 0 00	•			CABLOV	161	
	运输	#K11E (0.110)					CABUOY	1 92	
	116 70	MMA1 (/1 X . 30	MINITIAL	FORMATICITA, SOMINITIAL VALUES AT TOP OF CABLES	ABLES		CABUDY	193	
		(TE (6.117)	XA.YA.XPA	* YPA			CABUDY	161	
992	D# 711	18HAT (11K+34K	Az. F 18.4.	FORMAT (2K. 3.1XA = F. 10 = F. 4K. 5AYA = F. 10 = F. 5K. 4XXVA = F. 10 = F. 5K. 4XYVA =	WKKVA.F18.4	· SK· +HKAR.	CABUOT	567	
	16.7	16.10.4)					CABUDY	961	
	22 V +	45 MRITEROODS) MC	J				CABUOY	1 97	
	65 FD	FORMATCINISK. MEMBUR NUMBER. LS)	. MURKET.	UMBER.13)			CABUOY	16.1	
	Z Z	#R I T E (6.86)					CABUDY	208	
59;	04 99	KHAT (/104,2	6MSTEAUY -	ee formativida, 26 msteady - State Configuration)	2		CABUGY	202	
	**	ITE (6.64) T	IX.T.Y.O.	*			CABUOY	202	
	0 9 9 9	AMAT 1/1 K. LM	11x f 10.	2.5X.4HT LY = .F 10.2.	5x . 1 0HO I RECT	10N# .F 18.23	CABUOY	203	
	ž	#KITE (6.67)						584	
	67 FO	IRMA? LX. 4H</td <td>NODE . 1X . 9</td> <td>IS BEHIND.</td> <td></td> <td></td> <td></td> <td>202</td> <td></td>	NODE . 1X . 9	IS BEHIND.				202	
2	118	SHY STRIFT) * 3X * SHX (TIK - SHE CIR(FI) - OK-SIK(FI) - SK-SIK(FI) - OK-7ITER(LB) - OK-SIKDIN (OED)	PHTEN(LB) . 3X	. 9MPHIS (0EG)	CABUOY	282	
	ŞT	75×6					CABUOY	207	
		11E 16.691 J	S.SR.SE.X	HKITE 16,691 JS.SR.SE.XC.YC.TEM.PHISD			CABUDY	208	
		RHAT (I to 1 X.	6F 10.2)				CABUOY	588	
	WT 782	1447					CABUDY	210	
	-1	ILST = NCAB					CABUDY	211	
		101R.LT.3.3	ILSTANDO				CABUDY	212	
	203	COMPUTE STEADY-STATE CONFIGURATION	STATE CON	FIGURATION			CABUDY	213	
	8	71 ISINOIL	21				CABUDY	214	
•	21						CABUOY	215	
3	1 7	AF TO AKING ON A CONTRACT A		-			CABUDY	912	
							CABUOT	212	
	¥ 9.			これが日本のものである。 しつかたしまたい アンド・マングス・スピー・スピン・スピン・ステン・ステン・ステン・ステン・ステン・ステン・ステン・ステン・ステン・ステ			CABUDY	112	
	1	MPUL BACKED						226	
***	: ==	KF=TREF(11)					CABCO 4	200	
:								7 7 7	

	PROGRAM CABUDY	CA # CO #	12/12	0PT=0 A	OPTER ROUNDES TRACE	TRACE	FTH	FTH 4.6+428	04/19/17	11.49.22	
									,		
		7777777	1717						CABCOV	222	
		TEXIS	TEX1=1./C2(1C)	•					CABUOY	223	
		SEA CO	rc(10) /2	•					CABUDY	224	
		100	00 61 11:1.2						CABUOY	226	
05.2		78(1) #1EM	*TEN						CABUDY	326	
		2149 (2) 0Y	SING						CABUDY	222	
		79 (3) BXC	- XC						CABUDY	228	
		78(4) = 40	240						CARCO	220	
		35=(5)0%	# SE						CABUDY	2.0	
54.2		CALL	KUIMEA 15	. SR. 78. E	P'STA . SPA.	CALL RUIMEN (5.5%. 48. EPSTA, VPA, STAT, MCK, EPR, STAT)	TATE		CURR	3	
		FIRSTAL	*4.						CABUDY	2.32	
		TENST	D(11) .PHI	S=Y8 (2) &	XC=70(3)	TENETO (1) PPIS # 78 (2) 5xC=70 (3) 5YC=70 (4) 5SE = 70 (5)	3		CABUOY	2 53	
		PH150	PHISO= TO(21 - RAJ	t A D					CABUOY	2.34	
		11) JI	ER. 66.1)	2 01 00	50.				CABUDY	2.35	
300			1 154.4)	C.SR.SE.	MRITE (6.64) IC.SK.SE.KC.TC.TEN.P41SD	051+d•H			CABUOY	2 36	
		205 15 (11.	IF (I.1-1) 61./2.61	72.01					CABUOT	2 37	
									CABUOY	2.38	
		- FREN	TEMAS CLUS BILLING	_ ;					CABCO	23 3	
501		70767	TALLOUGH BY TALLOUGH	7					CABUDA	5 1 1	
•	ن	SOME CHE MOTOM TO THE STATE OF	TORY	SMA CMA	TA AT ACC	*			100840	142	
	•					-			20000	242	
			3	•					YOURAN OVERAN	242	
		15 (01)	4.17.0.3	16 (414.17.0.) 18:1C-1						***	
510		11 31	7 (8 3 7	[F(IM-15.8) GO TO 184						577	
		F 88 X # D	5.840.A	BSCCDABX	FORM D. SPRED BROKEDARK MEND OF BROKE COM	PASS (CB)			- CORC	246	
		16 (18.	. CG. NB3	IF (18.CO.NBJ) FBX=FBX+184	- LE -				CARUDY	44.	
		16 11 8	. £ 4. KB U	IF II B. E.L. NBUS URGATHEFAK	¥P J				CABUDY	542	
		14.3 F BYRH	F BY = HBU(LB)						CABUOY	250	
315		-	IN(PHI))	BLESIM(PMI J) . TEN+FOX					CABUOY	251	
		182=-(CONCPALS	82=-COS (PAIS) .TEN+F BY	<u>+</u>				CABUOY	252	
		IF (AU)	S([82) .L	1.0ELTA)	IF (Ausifdz) .LT.DELTA) T82=DELTA				CABUOY	253	
		I E NE N	URTITBE	T 61+ T 62+	182)				CABUDY	254	
•		72/12/	PAINTAIN AND AND AND AND AND AND AND AND AND AN	17 6-1 82)					CABUDY	5 2 2	
* > >		37784	344 A 744 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	, ,					CABUOY	952	
		Ž		* * * * * * * * * * * * * * * * * * *	.				X 600		
		16 (11	£4.68.13	IF (1164-68-13 GO TO 24	.				CAR:00	36.7	
			1 (69.9)	8.5K.SE.	HKERE (6.69) LB.SN.SE.KC.YC.TEN.PHISO	4, PHISO			CABUDY	250	
325		74 COMTINUE	MUE						CABUDY	652	
		THORK	THURKA-TERSTRIBLES	(STP4)H					CABUDY	560	
		TVLRT	* +1E N. CO	S (PMIS)	1				CABUOY	192	
		16 1171	1F 111Ex.4E.19 GO TO	50 TO 2	207				CABUDY	292	
***		NE STATE OF	MANAGE AND						CABUDY	563	
3				3 5 5 7					CABUDY	3 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
		3 T X P	#X11616,751 1V687	× 6 8 3					* C : 6 4 5	592	
		75 FORMA	1 6 1 K . 2 3H	M-COMPON	ENT OF TE	ENSTONA FIRE Z. 1X	. 2 ML B.		CABUDY	267	
		76 FORMA	1 (LX . 2 3H	Y-COMPON	ENT OF 16	'S FORMAT (1X.2 3HY-COMPONENT OF TENSIONS, FIG. 2, 1X, 2H(B)	. 2× 0		CABUOY	268	
335			TA.LE.O.	60 1C 2	11.				CORR	7	
	()		ENU COAS	ILIONS F	OR I KANS!	MIFTAL TO SUBROU	T SE ITERA	TERA	CABUOY	569	
		207 PHISLEPHIS	SIE!						CABUDA	276	
		AXL SKC	. ن						CABUOY	271	
•)	; د						CABUOY	2/2	
*				*********					CABUOY	273	
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			***************************************	7 2 7 4 7 7		(X)					COMMO	24.3	
		- 6	T SATABOLT								CARUDA	, X	
348		2	TAKE IN-D-5-PRO-COAS K-CCF (-ABS (CCF1)	HOCO	× 54	CCFICA	85 (C.F.1)				CO#.	105	
		*	17£ (6.56)	*		•					2000	901	
		0 g 95	FORMAT LAX, 25HREVISED WALUE	HREVI	33	VALUE	OF MIND LOS.F12. 4.1X.2KLB)	12. 4. 1X	. 2KL 8)		CORR	101	
		0,9	00 to 93								CORR	185	
		42 24	17 (ABS(PAZ)(1) > LT.360.) GU	11111	.360		10 45				CABUDY	912	
7 62		9	UO 83 121. MCAB	•							CABCOA	X 0 X	
		Z	AND CLUENT LOCAL	2							CABUDY	202	
			MICCOLLEGI	=======================================							CABUOA	542	
			CONT INCE								CABUDY		
;		0 :	COM1 1MUS								100043	162	
2.5		2 (262	
		200	20 FS 1F7 - 40FS	c							Y01470	2 2	
			N 1 1 2 2 4 2	70200		2 (3+30					CABUDY	Š	
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3.75		Z.	PELECTURESTOCT	10.01	007						CABUOY	29%	
		×	IN (1) #K.10* 41	IC . AFA(100	00.400	ARK(1) #K.10* AMC *AFAC *PI *0C* (DG / 4 .) *FLC ! () /FNI 1	I TN			CABUGY	298	
		3	CM(1) = CM(1) *# LC(1) / FN11	(1)33	FRZ	_					CABUOY	\$	
		Ŧ	ATC4 1) = AC (1) * FLG(1) / FM11	FL511	====	11					CABUDY	30 E	
		9	CONTINUE								CABUOY	70 K	
360		×	CONT.	# T 5		SC 2.1 / R.	OF C					5 0 1	
			TARIBURY (IN TOOK PRIDATE) (AND TO THE PRINCE OF THE PRINC	200	7	4×/(1)	3	1	1		¥ 6	2 (
	-		UIE A AND	1 6 A 3 E	5	AMALA	AND TENSTON	2016	F C 15 7 1		100000	7 6 6	
		200	1 = 1 - /62 (12)	8							CABUDY	-	
3.85		IF	(TENICIE)	E . b . 6	40.00	1) TEN	I (IC) *0.00000				COR	111	
•		E	TAC+ (TENI C	£	Š	2)/(2	THIRD (TENICLE) - YR OF (IC) /CLCC)				CABUDY	308	
		13	STR=ABS (THTAG) ** (C21 -1.) * FNTRC	3) (21 -1	. S FRT	28.0				CARUDY	346	
		T.	13 + CCT137 4 E	. +STR	_						CABUOT	387	
		×	XIIC) *XA-FL *SIN(PHIV 11C))	HATEL	2 2	ŝ					CABUDY	3 P	
200)	IC) = YA.F.L.	100	3	3					CASUDA	# B P	
		×	AP (IC) EXPI (IC) SYP (IC) EYPI (IC)	2 X P	3	TPICIC	•				CABUOY	a v	
		4 4	AAEX IC								CABOOA	717	
		: q		GOAS		Z A D / A C					9900		
202		4	CONTRACT.								CABUOY	6 PM	
;			NUE1 = 2º1140								CABUOT	430	
		Ž	NDE2 * MUEL								CABUDY	318	
		41	IF (FSM1.GE.2000.) NUEZ=NUE1+3	100.	1DE 2:	*NOET+	•				CABUDY	316	
		ď.	E = 2 - MUE 2								CABUOY	317	

	PMOGRAM GABUOY 74/74 OPT=8 ROUND=4/ TRACE FIN 4.6+428	04/19/77	11.49.22
9	11 ± 0	¥01840	
•	# 5 1 1 1 5 1 5 1 5 1 5 1 5 1 5 1 5 1 5		
	CONTRACTOR OF THE PARTY AND AND THE PARTY OF	2000	
		A COMMO	125
	ZZ FOMMAI (/1X+ 4MN ODE - ZX+ BHPMI (DEG) - 3X+ /MIEMIL 3) + 4X+ SMX (FI) + 5X+	CABUDY	322
405	1514 <f1>+4-614-614-614-614-614-614-614-614-614-61</f1>	CABUDY	323
	DO 36 Istanche	CABUDY	324
	· TaTT	CABUOY	325
	GO SETTERS. (S. V. DETOLES TERIOLES VILLES V	CABUDY	326
	2.0. FORMAN CAKAN 30. MAP 10. B. F. 10. 2. 2F. 10. 2F.	CARLOY	327
4.10		CARCOY	42
•	CECHPERCHE JERREN FELLE ES	YOU A	
	THE PARTY OF THE P	ACT 1	
	EXAMPLE CONTRACTOR CON		
	OO TORDANIA TANDA		127
•		CABUON	225
417	WAITE (6.25) TOTT-012		223
	25 FOWNAT (LZX+11HTDTAL TIME*+F10+4-1X+3HSEC+3X+10HTME STEP*+F10+4+		400
	11x+3HSEC)	CABUDY	335
		CABUDY	336
	2/ FORMATILM1.10x.42HCOMPUTED CABLE SYSTEM MOTIONS AND TENSIONS)	CABUOY	337
420	MX.1E (6.20)		336
	24 FORMAT (//ZK.SANUM.SK.6HT (SEG). 2K.7HDT (SEG). GK.5HHT).SK.		339
	ASTACRATION OF THE CONTRACT OF		9 5
	THE STATE OF THE S		14.1
			* * *
36.7			***
653		20000	?
		CABUDY	# () # ()
	A L LANGE AND THE STATE OF THE	CABCOY	345
	10010	CABGO	346
	A MENING A LINE OF THE PROPERTY OF THE PROPERT	CABCOT	367
430	00 Z41 44H1.7508	CABUOY	0 J M
	UD 161 1:1+RBO	CABUOY	848
	TO (fe III) BE (II) BE (Fe II + II)	CABUOY	350
	ADACA MIND SANDER OLD BE SANDER	CABUDY	352
	101 CONTINUE	CABUOY	352
4.55	1F(MUE2-EQ-NDE1) GO TO 114	CABUDY	353
	FO(ROL-5) HIS FO(NOE-4) HRPSISYO(RDE-3) HZETI	CABUOY	354
	76 (NUE-2) = 21P1578 (NOE-1)=571576 (NOE) = SYPI	CABUDY	355
	114 SF (TI-GL-TENE) OTHOT?	CABUDY	356
	IFICITAGE-IIMID AND. (FT.CE. IMZ)D STARTOD.	CABUDY	357
**	CALL KUIMER(NDE.11.YO.EPGTM DI.STARI.MCX.EPR.DYMA)	CORR	113
	C COMPUTE SUAFACE MAYE MOTIONS	CABUDY	359
	>CESCO PALSO PALSO P	CABUDY	366
	こののでは大きのできるので、 このののできるのでは、 このののできるのでは、 こののできるのでは、 こののできるのでは、 こののできるのでは、 こののできるのできる。	CABUDY	361
	KONFORO-PANKOONE	CABUOY	362
4 6 5	DO ACA MARK SEE	CABUDY	363
	MXSF BAKSA (E B. P. NS. L. C. L. B. P. T. T. P. T. L. F. T. F.	CABUDY	364
	CENNICENSIA DE CENNICE		365
	XSEE XXX F D X (X X) + C X X X X X X X X X X X X X X X X X X	CAGUDY	366
	STREET ST	CABUOY	167
4 50	XXXXII IXXXXII IXX AND	CABUDY	368
	LICKED CENT DEGREE CENT DEGREE CENT CENT CENT CENT CENT CENT CENT CE	CABUOY	369
	INVERSAL DESCENSE OF STREET OF STREE	CABUOT	3.70
		CABUOY	37
	TOUR CONTRACTOR OF THE PROPERTY OF THE PROPERT	CABUST	212
4	ACROMINATION OF THE STATE AND THE STATE OF T	CABUDY	373
	162 FORMAI (/1x.44.AVE.Fe.6.6.2F10.2.4F10.4)	CABOUT	* n

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	PROGRAM CABUCY YELVE CONT.	OPT = 0 ROUND= 1 TANCE	FIX 4.5.428	34/19/77	11.422
	IF (NUEZ-EG-NOE1) GO TO 121	20 10 121		CABUOY	3.75
	STI d/ "B\$T aTA>ADAS	1d/"BET-ddagsgddagastd/"Cet-1dagsgddagg1d/"Bet-1acacas	147.081.49		376
	MRI1 (6,123) TT.M	HAIII (6.123) TI . MCK. MSI. ZEII. MPSI. ZIPI . MPDSI. ZEIPP. SYD. SYPO. SYPO	I.ZETPP.SYO,SYPO,SYPPC	CABUOY	377
469	AZS FORMATILX . 4 HBUOT .	FORMAT (1X . 4 MBCOY . F & . 4 . F & . 6 . 2F & 0 . 2 . 4F & 0 . 4 . 107 . 2F & . 2 .	Fo2F B. Z.		378
	11X+5MSYPP#+F#+2+1X+4HD/5S)	K.4HD/SS)	•	CABUOY	3 79
	121 UELX#K(1)-KSBOELY#Y(1)-YS	54-(1)41		CABUOT	308
	SK-(T) GREANGESN-(T) AREANO	** (1) - YSP		CABUOY	3.61
	FLS#SQRT(O£LX*O£LX*O£LY*O£LY)	K+DEL Y+DEL Y)		CABUOT	292
465	FIOSHATAN2(-DELX, DELY) *140. /PI	3ELY) *160./PI		CABUOY	200
	EPS= (FLS-FLC(1)) /FLC(1)	rc (1)		CABUOY	305
	EPSP= (0E, X* 0XP+0E)	EPSP=(06,X*0XP+06,Y*0Y;)/(FLS*FLC(1))		CABUOY	305
	1EMS2=C111) + (4851)	1EMSS=C111) + (ABS1EPS) + + + ; ; > (1) -1., + eps + 1REF (1) + cimi (1) + Eps +	(1) +CIMI (1) • EPSP	CABUOR	386
	AFGTER-SOLC.TALK) TENSSTALL	7ENSS-TRIX		CARUOY	387
2,	F1P5=1(-0XP/CLS)+	f ips = ((-0xp// LS)+del x • (del x • dxp+del y • dyp) / Fls + + 3) /	12++3)/	CABUOY	998
	1COS(FIOS*PI/180.)			CABUDY	389
	147" u91 uSd1 JaS ud13			CABUDY	3.00
		*** ** ** ** ** ** ** ** ** ** ** ** **	S.F 105, FIPUS, FPS, EPSP	CABUDY	348
	125 FORMATICKELLO. B. b.	FORMAS (2X+X) 8-4-54-64-27-20-2-25-25-40-33X+130-2-2FB-2-2FB-5)	0.2.2F0.2.2F0.6)	CABUOY	285
4.75	00 197 I=2. KCAN			CABUDY	200
	OELX#X(I) - X (I-1>50ELY#Y(I)-Y(I-1)	JEL Y=Y(1)-Y(1-1)		CABUBY	300
	(1-1)dx-(1)dx4dx0	DEPRES (1) - KP(I-I) SOYPRYP(I) - TP(I-I)		CABUDY	368
	FLS=SQRT (DELX*OELX+OELY+DELY)	K+0£L Y+0£L Y)		CABUOY	396
	FIUSHATANZI-UELX-LELY)*180./PI	LELY)*180./PI		CABUOY	197
07,	EPS= (FLS=FL3(89) /7LG(8)	71062)		CABUOY	308
	130+4×0×130>×4543	EPSP		CABUSY	344
	TEMSS=CA17)+1ABSC	Temss-clit	(I)+CIM1(I)+EPSP	CABUDY	30,
	IF (TEXAS.LE.) TIS TEXASSINIS	SENS SHIME		CABUDY	10,
	#1#5×((-0XP/FL)+	/ () + 0 X	70.457	C 48007	¥ 02
487	1002/102016/02/01/20001			CABUDY	207
	FIPOS*FIFE 180 . JP.			CABUOT	103
	11=1-1			CABUDA	405
	MRATE (6.29) II.TT	MRATE (6.24)	.(II) . \PP(II) . YPP(II) .	CABUDY	904
	ATENSS, FIUS, FIPOS, EPS, EPSP	eps.eps#		CABUDY	404
\$ 40	29 FOUNT (ZK . I J. FB. t.	29 FOUNAT (2K n [3.8 B. 4. FB. 6.2F 10.2. h F1 B. 4. F 10.2.2 FB. 2.2 FB. 6)	2F8.2.2F8.6)	CARUOT	904
	107 COMTINUE			CABUDY	603
	SF(II.6E.1011) GO TO 1980	TO 1000		CABUDY	*1
				CABUDA	
				CABUOY	¥12
4.95	1909 CONTINUE			CABUOY	413
	401S			CABUOY	914
	043			CABUDY	415

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SUBROUTTIE STAT	TAT	14/76		SPIRE SOUNDED TRACK	JAR.	EK 3	278 4.0+4W		\$4. 23.77 21. 44. CL	
								CARUDY	416	
		SUBSCULLAR STRICTS TROOPEN	10 15K	7 70 07 4		****		CABUOY	417	
		COSSON/CERCH CHANGE WITH CIRC CLINE 1. AN	MACH - LA	15 . T. T	£ 3 R.F.	14: 10: 11 1		CABCUY	014	
		UNIVERSITY FIRESTONES OF THE	#(5) • DE	K(5)				CABUOY	614	
		コケードに対して、これをなったののなのなから、これでは、これには、これには、これには、これには、これには、これには、これには、これに		2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2				CABUDY	924	
		CALL CURSTINGS SCI	(*) · C X)	,				CABUOY	121	
		CHORMSCOS IF INCRES CH	3 - 1 / N > R	.				CABUDY	422	
		CLANA-SLEARFLANCK) - CA	W(2)	,		****	11-11-11-11-11	CABUSY	4.23	
		しい グロマンコスギング	N(1)-1X	43710174	1	Z = 1 = 1 = 1 = 2 = 2	SIXXBIADS/TIX (1)-1XF-1/Cit-1-	CABUCA	724	
	. ,	17 DER(5) BL. +5 TRM	2					CASUOY	454	
_		CACACACACACACACACACACACACACACACACACACA	101 2 4 4 101					CABUOY	924	
		THE PORTE OF THE PROPERTY OF THE	724 - XX	CHOKIN				CABUOT	427	
		DEX . 1 0 - 0 00 () 1 x (5) 2 1 x 10	35 (F X M L Z	The state of	:			CABUDY	924	
		OFR. (2) = (-F1-51 X (F1X (A)) + XPUT) / (-F (A) 1))	#3.47#TS-	14 × 1 × 2 × 1				CABUOY	624	
		0241 51a-5 TWIFT K(21) * 11ER(*)	1FT K(2))	* SER (*)				CARGOY	5.38	
•		DEXIVE ACOS (FINIS) & COER(5)	F.X.M.(2) 5.4	JER (5)				CABUDY	4.31	
		RE TURN						CABUBY	4.32	
		2								

SURROUTINE	E DYHA	74/74 OFT## POUND##/ THACL FTH 4.6+420	12781790	11. 49.22
-		SUBROUTINE JYMA(T.FT.OF) DINKRSION JE(106: FT(106) COMMON/MOL/AKSM(20).AYSM(20).FISH(20).WKSM(20).SIG(20).WSM(20).	CABURY	N 4 8 1
•		1FISH(ZO), KSM COMMON/MOSP/SEK(ZO), FRSM(ZO), WSM, HRR, COMMON/MO, JI TRIM, MCAG, MCEC, MOE1, OKUL, GALL, TBYNX COMMON/MO, CLI (SO: GZ (SP), LIRY (SO), URK (SO), DRT (SO), ANN (SO), GM (SG),	CORR CORR CORR CABUDY	6 + 51 \$ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
e		COMMONANDO/RELATION CONTRACTOR TO THE TRY OF TAX OF		- N.M.2 S. J 8 J. F J. J. J 8 J. F J. J. J. J
51		CORMONATOR DAY TO ADDITE TO THE STATE TO THE STATE OF THE		
•	•	OC (2011-1) = 1	CADECO CABECO CO	
5	•		CABUOY CABUOY CABUOY	
8	ų		00000000000000000000000000000000000000	2
s :	•		CABCOY CABCOY CABCOY CABCOY CABCOY	153888
• • • • • • • • • • • • • • • • • • •	1 T			ererer.
•		ONFORM TO THE TANK TO THE TOTAL TOTA		
x	<u>.</u> د	IF (AB): (FEND) - GE.TBYEKY TENB-TRYEKTK* FEND/ABSATRIA) DO -1 LO-1 LEGG GENE-ATENECO - GENERO) CALCULATE EQUATIONS FOR CORNER ELENENTS	CAB'10Y CAB'10Y CAB'00Y CAB'00Y	".

URROUTINE OTH!	7/7	OFFEE ROUNDED TRACE	** / TARE		FTN 4.6	4.6+420	4776773	11.49.22	
	4 CM - 80 - 04 - 81	14 01 03 11					>01443		
	CATANGER AND	21 - VI - V						7	
							CABUOY	6.4	
	4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1						×01640	604	
	TE (TC. FO. CMCA	ているのではない。 このとうから かいかまり・ひょうしん はんかいしょう かいかい かいかい かいかい かいかい かいかい かいかい かいかい かい	•				CABUDA		
	10121041		•				CABUDY	46	
	DELKHK(IC1)-1	DEL ERK(IC1) - K(IC) SOFLY-Y(IJ1)-Y(IC)	121)-Y(IC				CABUOT	495	
	FIVEATANZI-JE	I. M. DELY					CABUOY	96,	
	UXPEXP(ICI)-X	P (10) 80 (P=YP	14X- (107)	10)			CABUOT	164	
	OSFECOS (FIV) SSNF 15 IN (FIV)	SMF .S IN (FIV)					CABUOY	;	
	FLM#FLC(IC')						CABUDY	ţ	
	EPSB= (DELY/CSF-FLM)/FLM	1F-FLN) /FLN					CABUDY	23	
	CPP8= (DEL Xº U)	IN OUEL YOU YPE	CONT. COEL	X++2+0EL Y++	2) • FLN)		CABUDY	70 S	
	7 Dac 1 (161) + A	12 . E P.S.B) ** (C2	(101)-1.)	·EPSB+TREF (1010+		CABUOY	205	
	TOTAL (ICT) -EMIST						>00 E 50	210	
	TYPETAPONER						CABUOY		
	TYB-TA-CSFA-TB-CSF	8.CSF					CABLOY	25	
	KEP. O. STEP. D.						OABUDY	215	
	00 37 IS-1-NSH	=					CABUDY	205	
	CK VBEKSE(IS)	, Y (12)					CABUDY	2.5	
	IF LASS CUKY) .	E.5.1 60 TO	20				CABUDY	510	
	11011110111111111111111111111111111111) 51 S - () 7 S - ()	12)•;•(SI	#(IS)			¥0140	111	
•		3, (27) 97 27 13	0 C X X T T T W X	CACANOMIC				776	
		107.1					CABUOY	216	
•		(31)					CABUDY	2 2 3	
	VRVA TEP- TP - EC						CABUOY	516	
	KRISORT (KRK. L	RX . VRY . VRY					CABUOY	213	
	VRMA VRK C.S.F.A	I + VRY + SMFA					CABUDA	510	
	- COLUMN TO COLUMN	VAYOSIE					CABUOY	616	
	MALA BANK SAN	A +VRY-CSFA					CABOOT	225	
			*****				>00 E 4 0	253	
	THE CASE OF THE PARTY OF THE PA	A CALL DO A CALL	1				CARUDA	223	
	07A=B. 9+0AT (IC) + VATA + ABS (VATA)	C) - VATA - ABS (VRTAS				CABUDY	\$26	
	010-0. PURT (1	C11 . VRT B. ABS	(VRTB)				CABUDY	\$2\$	
	DMX=FCA+DMA+C	SFA+FCB+DMB+	CSF				CABUOY	926	
	CHARFCA-DRA-S	Mr At Cae Dige	286				CABCOT	126	
	OTX = FCA OTA						¥000 ¥	226	
	TE A CEDABK ALL		COVERCED	.LT. 8.33 60	10 01		CABUDY	536	
	COAR-COABK! 10	:) /(COVOY(IC)	.0.10000				CABUOY	531	
	IP ((CDAR.LE.	IP ((COAR-LE.D. 5).OR. (CUAR-GE.Z.)) GO TO 71	. GE . 2 . 3 3	60 70 71			CABUDA	225	
	0000 00000	OADK (IC) • VR•	× * * * * * * * * * * * * * * * * * * *				CABUDY	200	
	CAY 8 8 - 7 A B C - C - C - C - C - C - C - C - C - C	COAST COTTE						4 0 0 10 0 10 0	
	er mideau eus sansadassidu sarn usuur un teeu samenoo	2000		TATE ROP CT		75.6	>00 E E	418	
ۍ د د د	CONTROL CARGON	TO AUDIC MANY	-7.26	בשנים בחצ כדי	K 10.	461	CABUDY	537	
-	IF (CUARY (12)	LT.6.) 60 TO					CABUOT	284	
5	ULAN-SORT (-4.	. COA BX(IC) / (1PID				CABUDY	\$ 39	
	IF (ABS (XPP I IC	333.LE.0.0000	081) XPP(101 - 0. 00000	1		CABUDY	240	
	WSAD=ABS (KP(IC) * KP(IC)) '(KPP(IC) * OIAN)	ICO . Kb(IC)).	(XPP(10)	OLAND			CABCOY		
	34243 (MDS=2AS		3 - 673736	2000	1130		CABION	245	
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		ŝ	CIVESON (-6. COASY (C) (1. 12. PI)	CABUDY	217
			If (AES/VPP(10)) . Le. B. Bebeckly VPP(10) : B. Bebeckl	CABUDY	215
			4240=485(4761C)-46(1C))/(486(1C)-01A1))	CABUOY	549
			SAZ85QRT (V2AD)	CABUOY	5.50
128			IF(>>2.6E.JKUL) SYZ=DKULSIF(SYZ.LE.OKLL) SYZ=OKLL	CABUDY	551
			CDABY(IC) = ((PI = DIA n = = 2/4) + 2 = 2 / 5 / 2 / 2	CABUDY	552
			ARECIO) HARBING PATE OF SKIP STOOL STAFF	CABUDY	553
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		ļ.	IN CHO. MO. NOC.) PIX: 1.4+10H	CABUDY	25.5
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			THE TRACT OF THE TRACTOR OF THE TRAC	CARUOY	
		•	AFOR ARELIC. COSTO + KABRACIO)	CABUDY	200
1.30			FIVES. S. IFUA CELLO + FC S. CELTOL + FCA CARTICO - SEFA CELEA	CABUOY	202
		-	IFCE+AMKICL) + SMF+SMF) + YMB-KIC)	CABUOY	296
			FKKE+0.50 LFCA0ARKIDOOSWFA0CSFA+FCS0AMKICLOOKFOCSF)	CABUDY	263
			HPP(1C)=(F1Y0FX-FKK0FY)/(F1K0F1Y-FKK0FKK)	CABUDY	200
			大学をこれのこのできまれているとのではしていかにはものにとしてあれるのではない。	CABUOY	5 6 5
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		?		CABUDY	260
			IF (FSH). SE. Zees. CO TO SE	CABUOY	\$69
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7	_	5 :		CABUOY	571
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				CABUDY	574
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			LF(FSW1. LE. 3000.) GO TO 41	CABUDY	205
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;			DE (NDE) » (BVXX • (FSY +FRARS) +PSIC • (FX+FRARX)) / (BVXX • (BIX+PS2) -	CORR	1 33
170		-	195160023	COR.	40.1
			JE (NUE-2) = (F1+FRKRY) / BVM2	CABUDY	297

	67 ts	403	136	137	30.00	ij	1 30	-	5	3	6.12		710	761		
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-		w. O	SUBROUTIAE SPECT Common/Mosp/Asmize) . Frsmize) . NSW. HTRC	PECT SM(20)	#8# ·	(20)	NSW. MTRC		CABUOY	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	
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2	_	1	FRYBER System (2 - 0 f) System (2 - 4 / Mrs - 6 / Whoso Abelly Bor (5 Se Ofline) Setion	SSE DEL	. (9 K)	•					
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I MICES MICH		ISM(20).A75N(20).FISM(20).MKSM(20).SIG(20).WSM(20)	CABUOY	671
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TY INTELE)		AEAD (5.1) X31.2ETI.SYOI.XPSI.2TPI.SYPDI	CABUOY	677
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### ### ##############################		121PI.SYPUI	CABUOY	699
			CABUOY	9 6.9
### ##################################			10000	15.0
UMBUIL COEFFICIENTS FOR SPAR BUOY dww.x.2**edws-175/6 dww.x.2**edws-175/6 UWRIL2**edws-175/6 UWRIL2**edws-175/6 UWRIL3**edws-175/6 UWRIN3**edws-175/6 UWRIN3**e	•		CABUDY	86.9
BUMER = 2.0		MPUTE COEFFACIENTS FOR SPACE	CABUDY	96.9
GABUOY GABUOX G		9/8###5.•6#5+T#5/6	CABUDY	\$6.9
10.11 [11.41.0] 10.11 [11.41.0] 10.11 [11.41.0] 10.10 [11.41.0		RHO/3.1 * ((BMS+TYS/C)/(RHG*P[*AYSM(NSM))) **1.	CABUDY	\$
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	•		CABUDY	700
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		JD 2 15=1+N3M2+2	CABUOY	202
			CABUOY	202
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AORRED (S.99%.ed]++1		42=445M(15+62+441=445M(15+12+42M(15)	CABUDY	407
		ACCENTACION AND ACCORDANCE	CABUOY	60.
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35	SUBROUTENE BUDY	1.17.		OPING MOULDAY/ FRACE	FRACE	X .	FTN 4.6.428	04/19/77	11.49.22	
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sh •	* 2	###17E(%-24) (958(1) - 1=1.MSM) ###################################	050(1).1 051(1).1 08.1271 08.1271 118.1271 1772.765	1.25 1.25 1.25 1.56 1.56 1.56 1.56 1.56 1.56 1.56 1.5	2F 10.51				~ ⊕ # □ ⊕ めいのます ~ ~ ~ ~ ~	
.	6 40 6 40 1 40 1 40	29 FORMATILESSMENTS FIG.5.54.5MRCSO.F10.5) GO TO 461 61 CONTINUE COMPUTE COUFFICIENTS FOR PROLATE OR OBLATE OFT-AYSMENTS BORSMENT	UNZ",F11	-5-54-51 R PROLAT	9 FORMAT(LK-STMANZ»-F10-5-5K-5MKGSO»-F10-5) GO TO 461 GONTONE CONFISCENTS FOR PROLATE OR OBLATE SPHEROLO OFT-AYSMALL BENERKSW(1) APPREXAMENTE	4ER010			\$17713 \$1775	
\$		F (HA-61.0-1) 60 TO FKS=0.0740*HA/0-1 FKS=1.2-04-0-1/HA FKP=1.27*0-1/HA	2	1 • 1						
• • •	:	-	60 TO 10 1	.61 H (3) = 0 H (7) = 1 H (1) = 5	1611(4) = 0 . 5 FII (4) = 0 . 5 FII (4) = 1 . 7				2000	
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		FH(13) =0.020 FP(1)=1.276F	(2) • 6.5!	SFP(3) =	FM(13)=0.020 FP(1)=1.276fP(2)=0.558fP(3)=6.3128fP(4)=0.117	.1		000 000 000 000 000 000	173	

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115		FP(5)=0.03568FP(6)=8.00388FP(7)=8.5FP(8)=8.0731 FP(9)=0.272sFP(10)=8.9936FP(11)=3.698FP(12)=8.28	000 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	725 97
		FP(13) = 20.0 FPS(1) = 0.35 FPS(2) = 0.26 FPS(3) = 0.23 FPS(4) = 0.23 FP	2 C C C C C C C C C C C C C C C C C C C	2 2 2
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